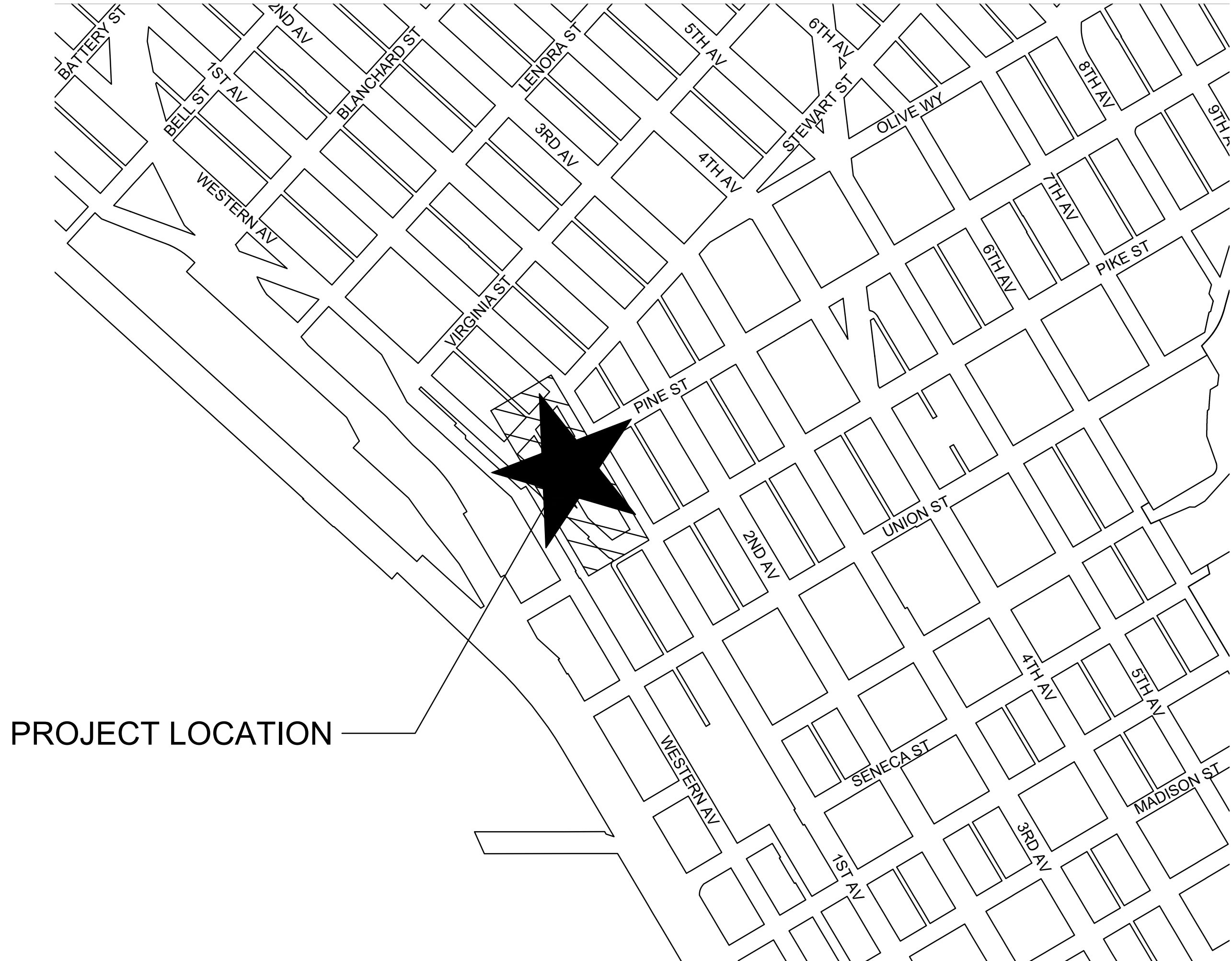


PIKE PLACE MARKET BALCONY DECK REPLACEMENT

PROJECT ADDRESS: 1518 PIKE PL, SEATTLE, WA 98104

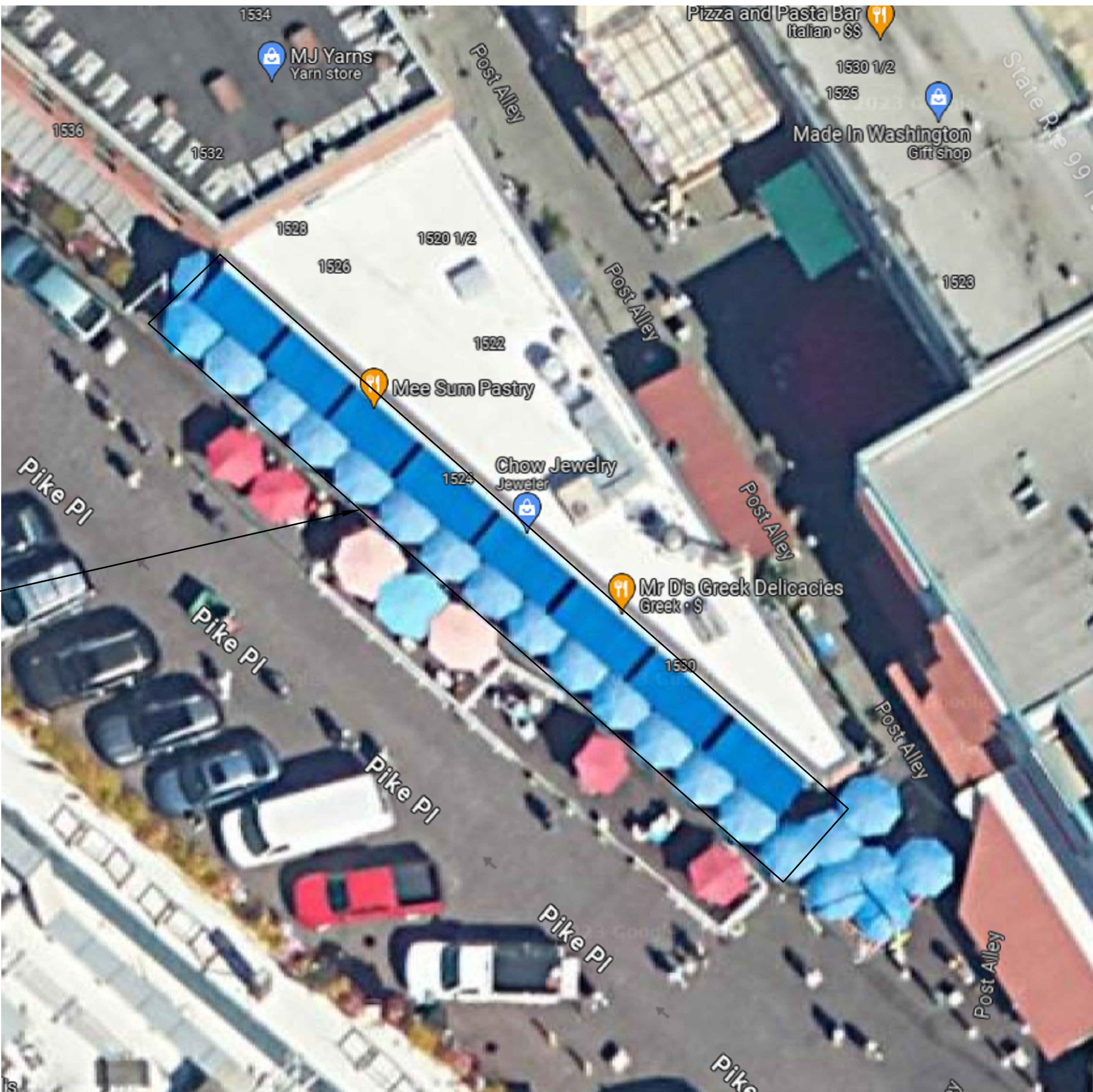


PROJECT LOCATION

VICINITY MAP
NOT TO SCALE



PROJECT
LOCATION



LOCATION MAP

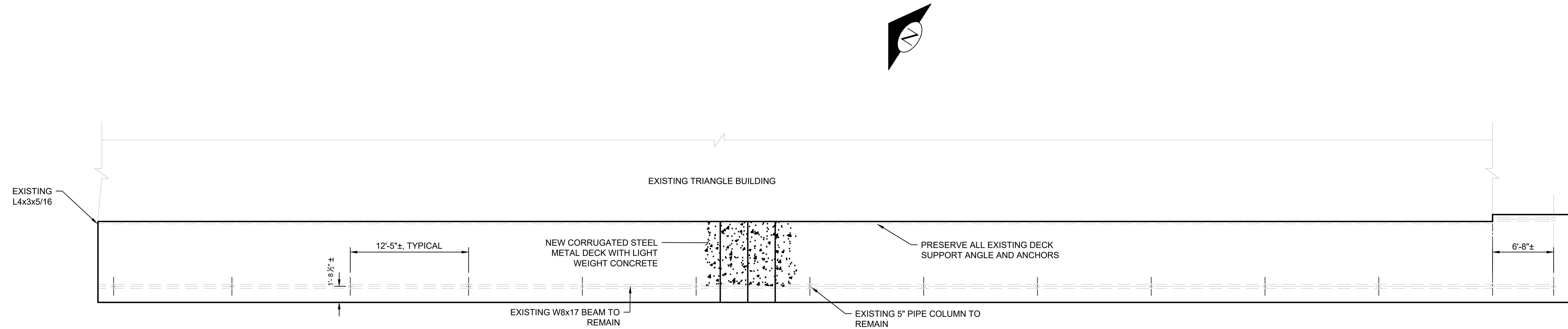
SCOPE OF WORK

REPLACE DECK CONCRETE AND CORRUGATED
STEEL WITH NEW. REHABILITATION OR REPLACE
HANDRAIL. HAULING OF REMOVED MATERIAL.

SHEET INDEX

- 01 COVER SHEET
- 02 PLAN
- 03 SECTION DETAILS

ONE INCH AT FULL SIZE	DESIGNED JL	7			RHC ENGINEERING WWW.RHCENGINEERING.COM 720 3RD AVE SUITE 1400 SEATTLE WA 98104 PH: 206.623.5984		PIKE PLACE MARKET	TRIANGLE MARKET BUILDING	DATE 11/20/2023
	DRAWN TL	6						BALCONY DECK REPLACEMENT	
	CHECKED GMC	5						COVER SHEET	
		4							SHEET 1 of 3
		3							
		2							
		1							
SCALE X		NO.	REVISION	DATE					



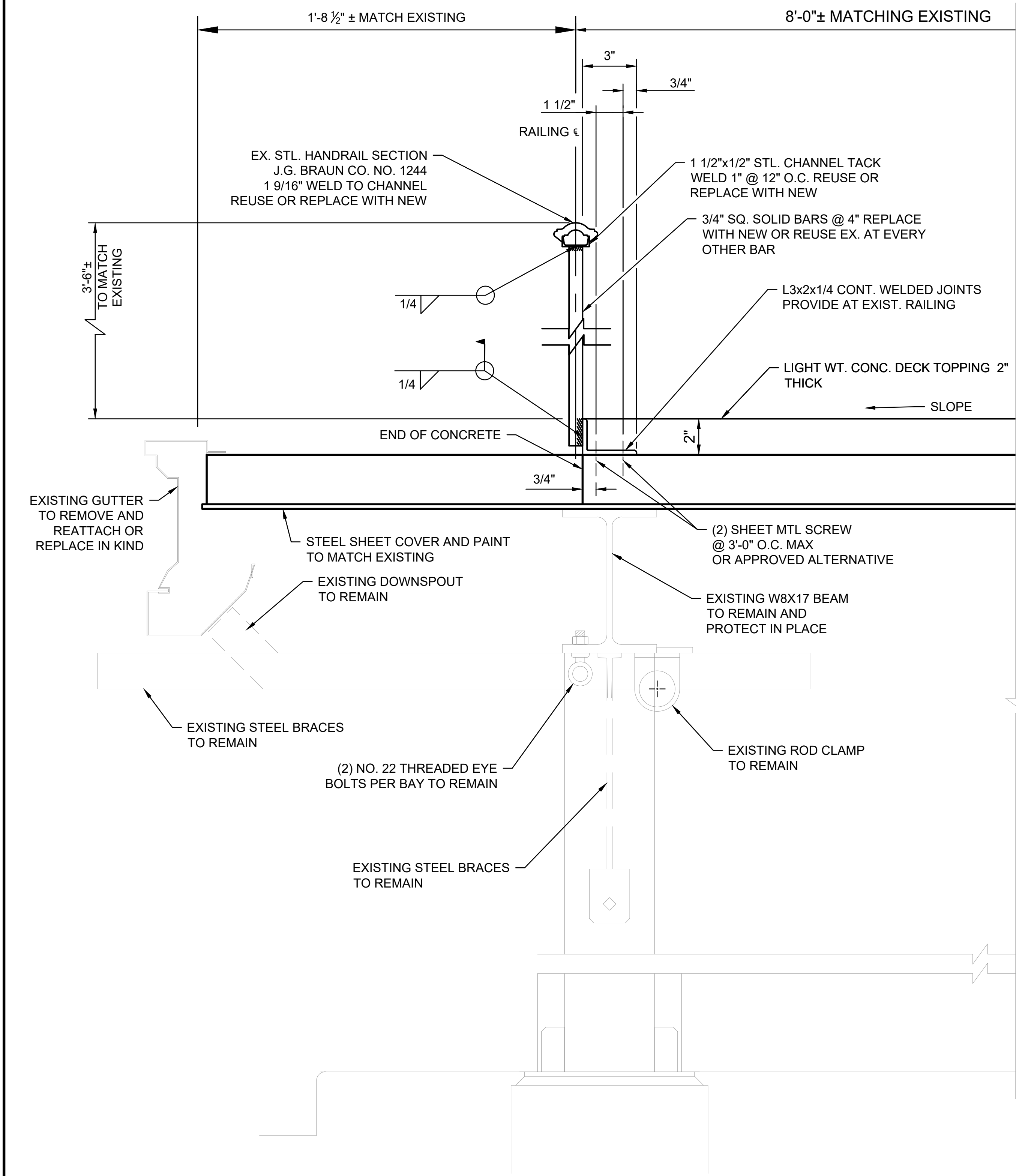
BALCONY PLAN

MAJOR MATERIAL LIST

- 1. CLASS 3000 LIGHT WEIGHT CONCRETE: 14 CUBIC YARD.
- 2. GALVANIZED WIRE MESH: 1 ½" 17 GAGE
- 3. 3 FEET WIDE CORRUGATED STEEL PANEL: 1600 SQUARE FEET, GALVANIZED AND PAINTED TO MATCH EXISTING COLOR FOR EXPOSED PORTION.
- 4. BOTTOM STEEL COVER SHEET, GAGE 22, 1600 SQUARE FEET, GALVANIZED AND PAINTED TO MATCH EXISTING COLOR
- 5. ASTM A36 STEEL ANGLE, L 3X2 ½": 160 FEET, GALVANIZED AND PAINTED TO MATCH EXISTING FOR EXPOSED PORTION.
- 6. STEEL HANDRAIL: 160 FEET, GALVANIZED AND PAINTED TO MATCH EXISTING COLOR.
- 7. CONCRETE DECK JOINT FILLER: 220 FEET.
- 8. METAL PAINT

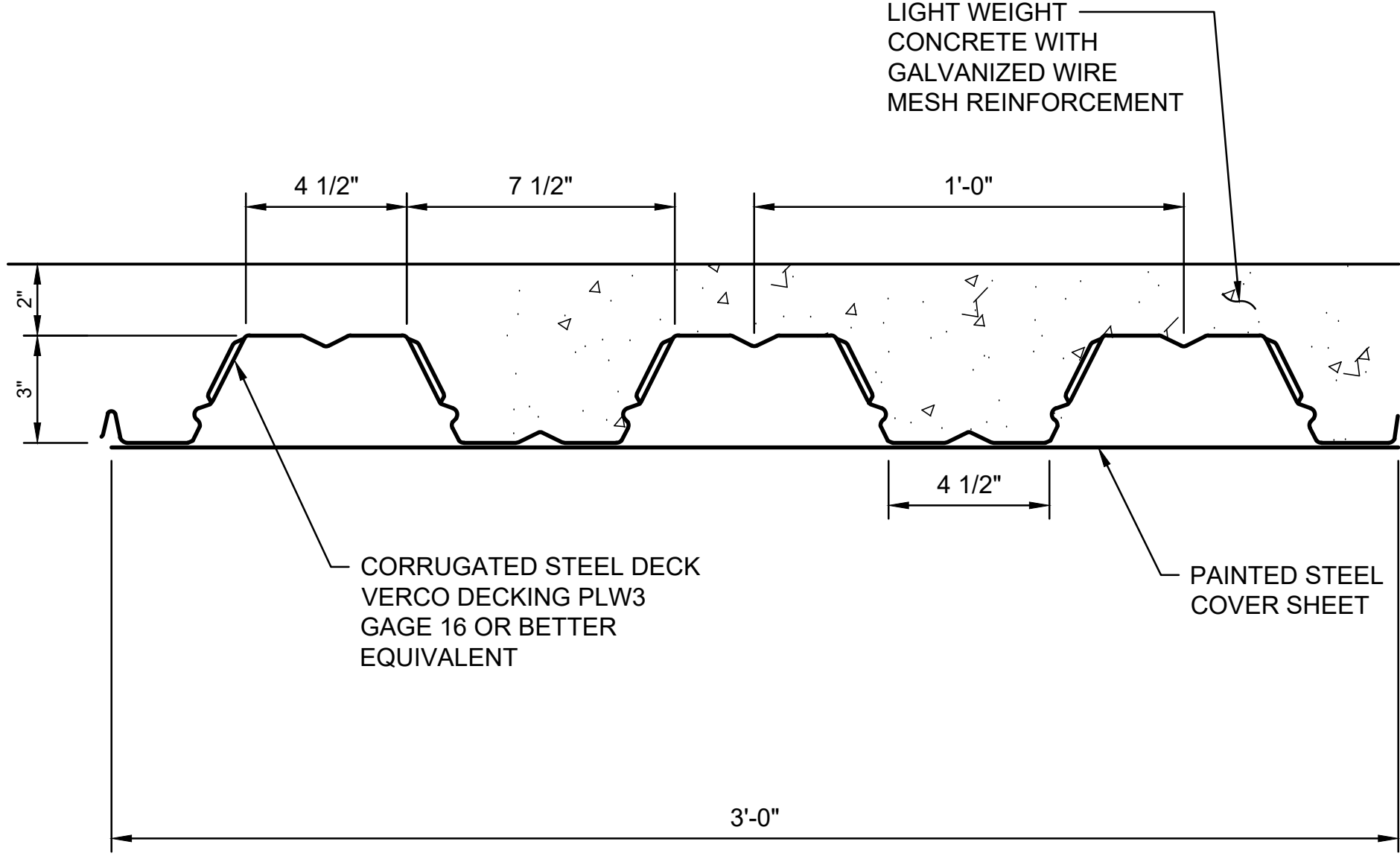
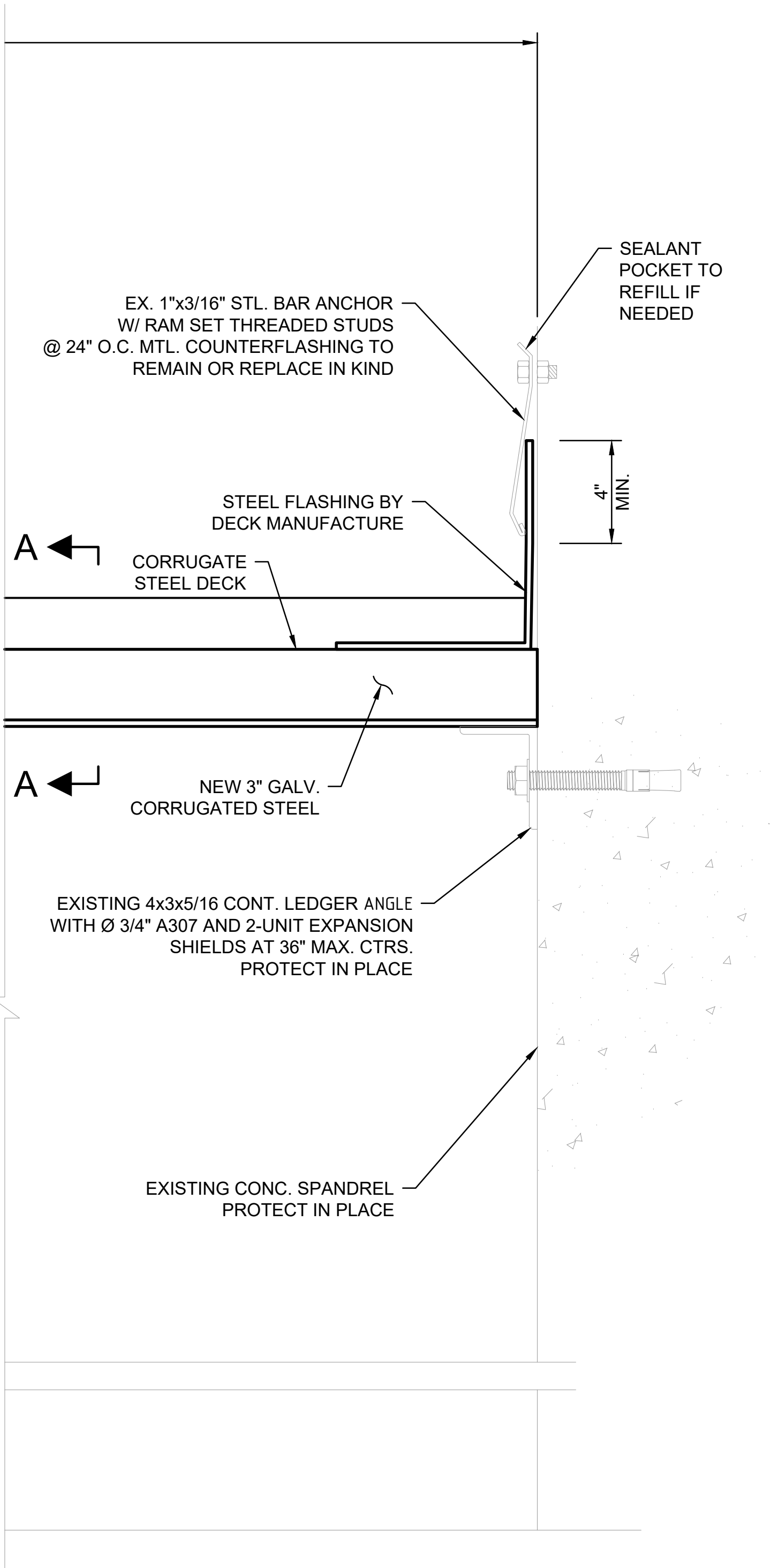
GENERAL NOTES:

- 1. ALL WORK SHALL BE IN ACCORDANCE WITH THE CITY OF SEATTLE STANDARD SPECIFICATIONS FOR ROADS, BRIDGES, AND MUNICIPAL CONSTRUCTIONS, 2024 AND THE CORRUGATED DECK MANUFACTURE'S RECOMMENDATIONS.
- 2. CONTRACTOR MUST VERIFY ALL EXISTING DIMENSION AND CONDITIONS.
- 3. CONTRACTOR MUST PROTECT ALL EXISTING BUILDINGS, STRUCTURES, AND UTILITIES UNLESS NOTED FOR REMOVAL.
- 4. SEE SHEET 3 FOR DECK REPLACEMENT DETAILS.
- 5. DECK CORRUGATED STEEL SHALL BE GAGE 16 OR BETTER EQUIVALENT AND COMPLY WITH ASTM A653/653M.
- 6. DECK CONCRETE SHALL BE CLASS 3000 LIGHT WEIGHT CONCRETE OR BETTER EQUIVALENT IN ACCORDANCE WITH ASTM C330.
- 7. DEKC EXPANSION JOINTS SHALL BE 1/4" WIDE AND 1" DEEP FILLED WITH POURED POLYURETHANE JOINT SEALER CONFORMING TO FEDERAL SPECIFICATION TT S00227E TYPE 1 CLASS A OR TYPE II CLASS A. THE JOINT SPACING SHALL BE 6 FEET.
- 8. HANDRAIL SHALL HAVE PAINT PRIME COAT AND BLACK TO MATCH EXISTING.



BALCONY TYPICAL SECTION

3" = 1'-0"



SECTION A-A

NOTE:

1. SEE SHEET 2 FOR GENERAL NOTES.

ONE INCH AT FULL SIZE	DESIGNED JL	7		
	DRAWN TL	6		
	CHECKED GMC	5		
		4		
		3		
		2		
		1		
SCALE 3" = 1'-0"		NO.	REVISION	DATE

RHC ENGINEERING

WWW.RHCENGINEERING.COM

720 3RD AVE SUITE 1400
SEATTLE WA 98104
PH: 206.623.5984



PIKE PLACE MARKET

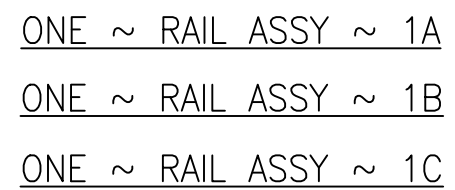
TRIANGLE MARKET BUILDING

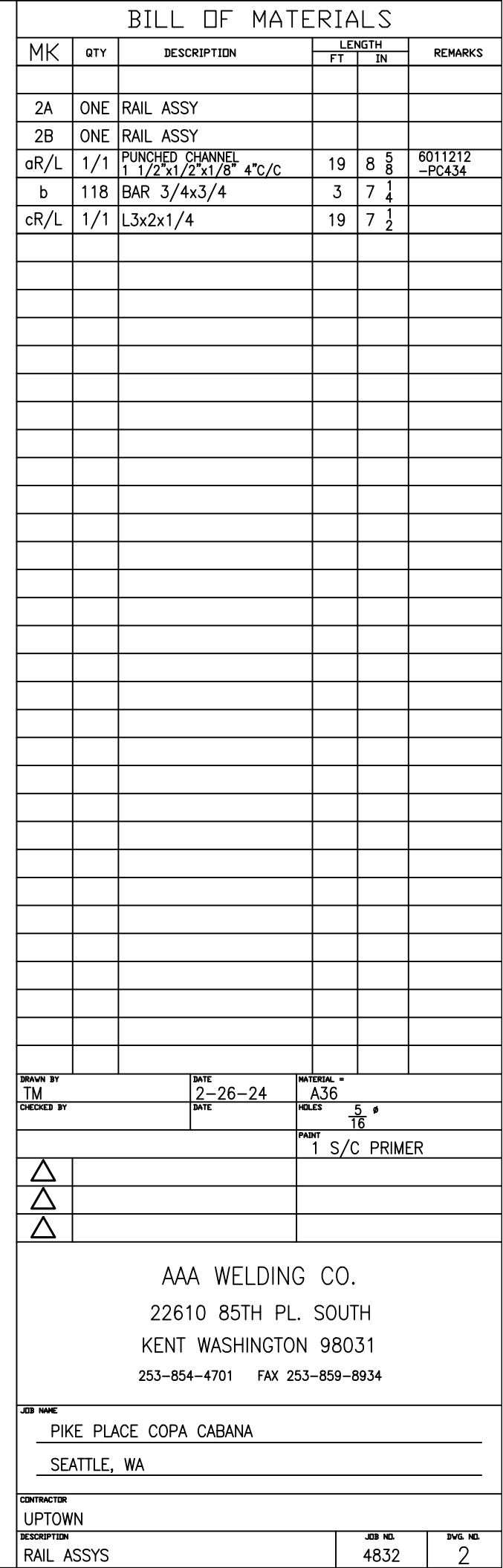
BALCONY DECK REPLACEMENT

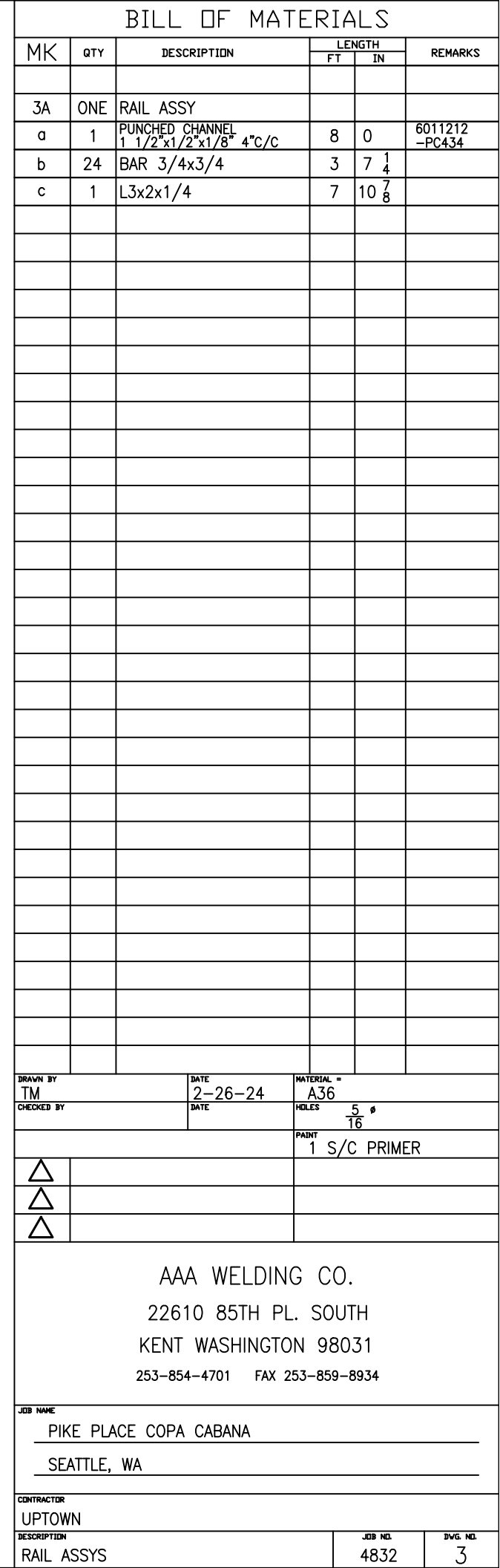
SECTION DETAILS

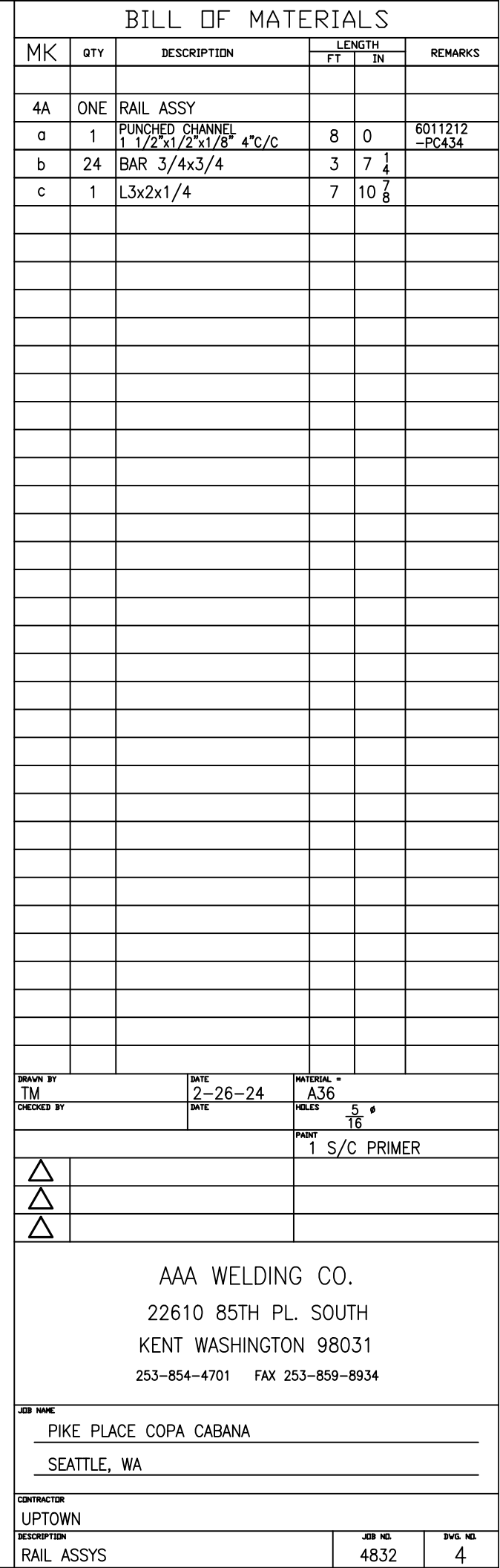
DATE 11/20/2023

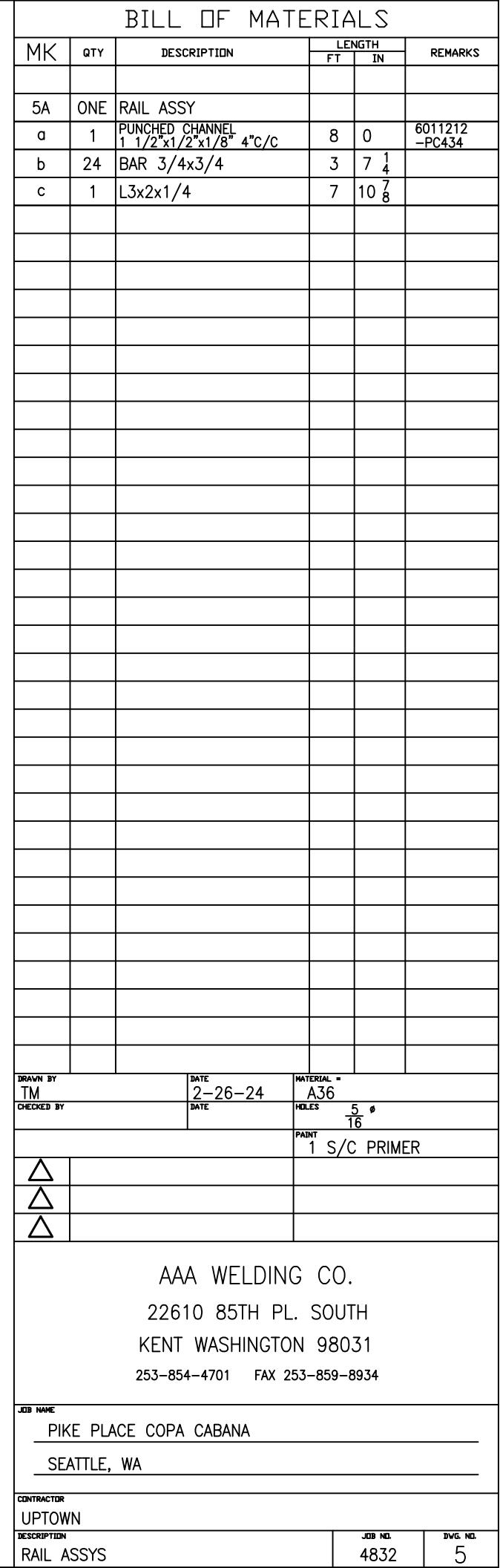
SHEET 3 OF 3

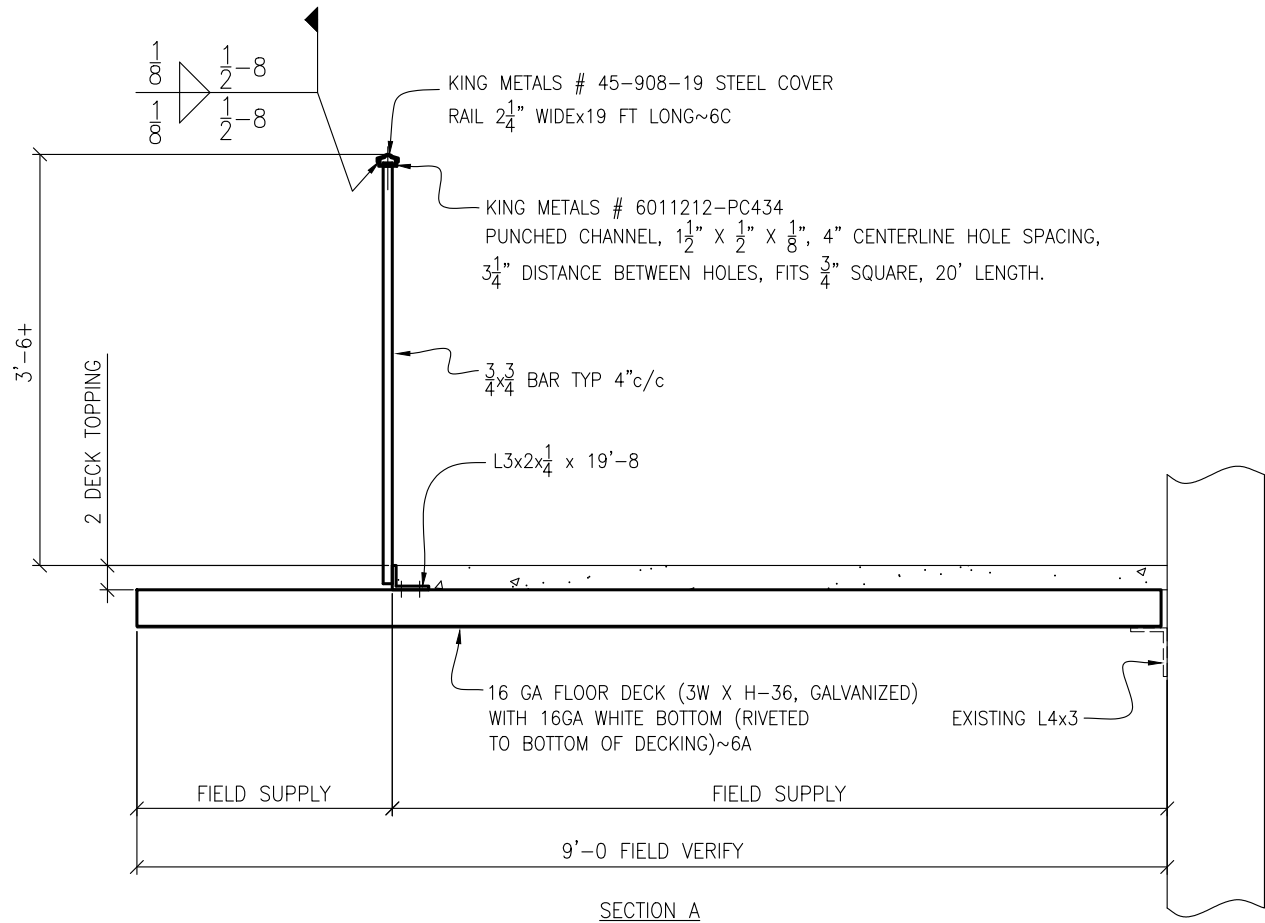
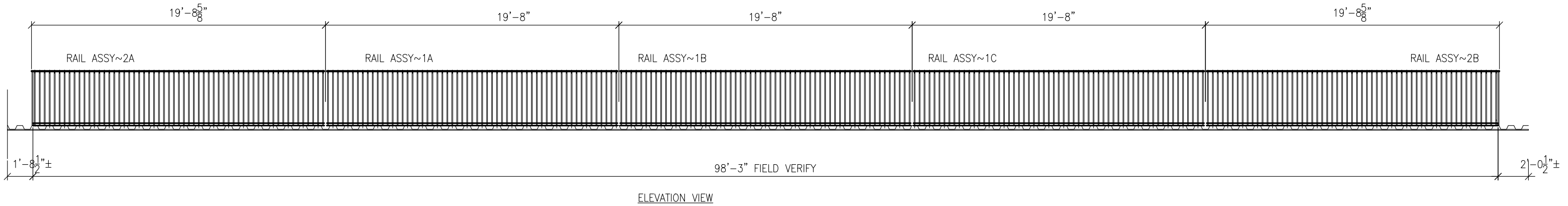
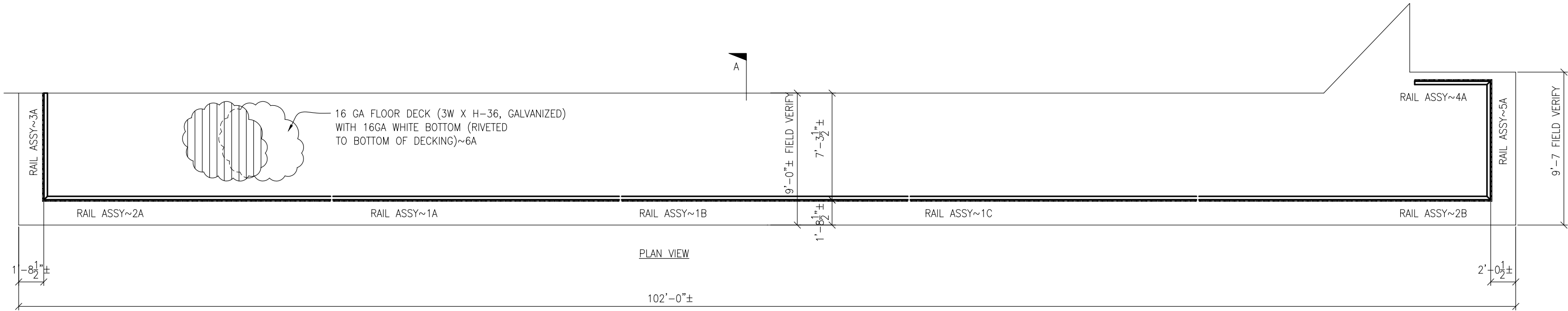
[illegible]











DRAWN BY TM	DATE 2-26-24	MATERIAL =
CHECKED BY	DATE	HOLES Ø
		PAINT
△		
△		
△		
AAA WELDING CO. 22610 85TH PL. SOUTH KENT WASHINGTON 98031 253-854-4701 FAX 253-859-8934		
JOB NAME PIKE PLACE COPA CABANA		
SEATTLE, WA		
CONTRACTOR UPTOWN		
DESCRIPTION RAIL PLAN	JOB NO. 4832	DWG. NO. 1















Project Submittal Summary

Project Name: Pike Place

Project Location: Seattle, WA

Customer Name: AAA Welding

Date of Submittal: Tuesday, November 19 2024

Your submitted project parameters:

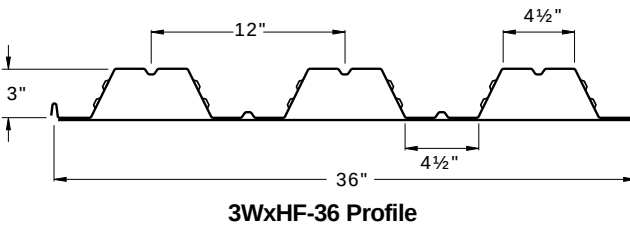
- **Panel Type:** Floor Panel
 - **Size:** 3" Composite Floor Deck
 - **Product:** 3WxHF-36 5" NWC (145 PCF)
 - **Gage:** 16/16GA.
 - **Attachments:** ATTACHMENTS: 36/4 AT SUPPORTS
-

Structural Capacities and Data for Your Project

Based on your chosen specifications, the structural capacities and data for your project may be found in the attached ASC Steel Deck catalog pages, which include actual data from the IAPMO report.

Liability Statement

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Panel Properties

Gauge	Weight	Base Metal Thickness	Yield Strength	Tensile Strength	Gross Section Properties				
					Area	Moment of Inertia	Distance to N.A. from Bottom	Section Modulus	Radius of Gyration
	w psf	t in	F _y ksi	F _u ksi	A _g in ² /ft	I _g in ⁴ /ft	y _b in	S _g in ³ /ft	r in
20/20	3.69	0.035 / 0.036	50	65	1.054	1.542	0.91	0.712	1.209
20/18	4.16	0.035 / 0.047	50	65	1.193	1.640	0.81	0.723	1.172
20/16	4.68	0.035 / 0.059	50	65	1.344	1.727	0.74	0.732	1.133
18/20	4.35	0.047 / 0.036	50	65	1.253	1.932	1.02	0.934	1.242
18/18	4.83	0.047 / 0.047	50	65	1.392	2.058	0.93	0.949	1.216
18/16	5.35	0.047 / 0.059	50	65	1.543	2.172	0.85	0.962	1.186
16/20	5.03	0.059 / 0.036	50	65	1.45	2.309	1.1	1.155	1.262
16/18	5.51	0.059 / 0.047	50	65	1.593	2.457	1.01	1.174	1.242
16/16	6.03	0.059 / 0.059	50	65	1.744	2.595	0.94	1.191	1.220

Gauge	Effective Section Modulus at F _y					Effective Moment of Inertia for Deflection			
	Compression	Bending				Moment of Inertia		Uniform Load Only	
	Area	Section Modulus	Distance to N.A. from Bottom	Section Modulus	Distance to N.A. from Bottom	Moment of Inertia	Moment of Inertia	I _d = (2I _e +I _g)/3	
								I ₊	I ₋
	A _e in ² /ft	S _e + in ³ /ft	y _b in	S _e - in ³ /ft	y _b in	I _e + in ⁴ /ft	I _e - in ⁴ /ft	I ₊ in ⁴ /ft	I ₋ in ⁴ /ft
20/20	0.481	0.538	0.75	0.645	1.42	1.506	1.186	1.518	1.305
20/18	0.551	0.540	0.66	0.674	1.23	1.582	1.363	1.601	1.455
20/16	0.650	0.562	0.61	0.701	1.04	1.640	1.512	1.669	1.584
18/20	0.691	0.875	0.98	0.844	1.49	1.930	1.484	1.931	1.633
18/18	0.761	0.878	0.88	0.879	1.34	2.056	1.693	2.057	1.815
18/16	0.860	0.875	0.80	0.910	1.19	2.170	1.868	2.171	1.969
16/20	0.923	1.175	1.11	1.047	1.53	2.306	1.790	2.307	1.963
16/18	0.997	1.194	1.02	1.084	1.40	2.454	2.012	2.455	2.160
16/16	1.095	1.211	0.95	1.119	1.28	2.592	2.214	2.593	2.341

Reactions at Supports (plf) Based on Web Crippling

Gauge	Condition	Bearing Length of Webs							
		Allowable (R _n /Ω)				Factored (ΦR _n)			
		1"	2"	4"	6"	1"	2"	4"	6"
22	End	296	368	471	550	452	564	721	842
	Interior	522	630	783	900	776	937	1164	1338
21	End	379	470	598	697	580	719	915	1066
	Interior	667	801	990	1135	993	1191	1472	1688
20	End	424	524	666	775	649	802	1020	1186
	Interior	746	893	1101	1261	1110	1329	1638	1876
19	End	600	737	930	1078	918	1127	1423	1650
	Interior	1054	1252	1532	1747	1568	1863	2280	2599
18	End	743	908	1141	1320	1137	1389	1746	2020
	Interior	1305	1542	1878	2136	1941	2294	2794	3178
16	End	1143	1383	1723	1983	1749	2116	2636	3034
	Interior	2008	2350	2834	3206	2986	3495	4216	4768

Web Crippling Constraints

h=3.2"

r=0.125"

θ=63.5°

2.3 3WxHF-36 Cellular Composite Deck

Normal Weight Concrete (145 pcf)

Maximum Unshored Spans



3WxHF-36 Cellular Shoring Table

Total Slab Depth (in)	145 pcf Normal Weight Concrete					
	Volume cy/100sf	Gauge	Slab and Deck psf	Maximum Unshored Span		
				Single	Double	Triple
5	1.080	20/20	45.9	11'-11"	13'-6"	12'-9"
		20/18	46.4	11'-11"	13' - 10"	12'-9"
		20/16	46.9	12' - 3"	14' - 2"	13' - 1"
		18/20	46.6	13' - 11"	15' - 5"	13' - 4"
		18/18	47.0	14' - 2"	15' - 9"	13' - 4"
		18/16	47.6	14' - 5"	16' - 1"	13' - 4"
		16/20	47.2	14' - 10"	17' - 7"	13' - 4"
		16/18	47.7	14' - 7"	17' - 3"	13' - 4"
		16/16	48.2	15' - 0"	17' - 9"	13' - 4"
6	1.389	20/20	58.0	10' - 11"	12' - 5"	11' - 9"
		20/18	58.5	10' - 11"	12' - 9"	11' - 10"
		20/16	59.0	11' - 3"	13' - 1"	12' - 1"
		18/20	58.7	13' - 2"	14' - 3"	13' - 4"
		18/18	59.1	13' - 5"	14' - 7"	13' - 4"
		18/16	59.7	13' - 7"	14' - 10"	13' - 4"
		16/20	59.3	14' - 1"	16' - 3"	13' - 4"
		16/18	59.8	13' - 10"	15' - 11"	13' - 4"
		16/16	60.3	14' - 2"	16' - 5"	13' - 4"
6½ 1 Hour Fire Rating	1.543	20/20	64.0	10' - 6"	12' - 0"	11' - 4"
		20/18	64.5	10' - 6"	12' - 4"	11' - 5"
		20/16	65.0	10' - 10"	12' - 7"	11' - 8"
		18/20	64.7	12' - 11"	13' - 9"	13' - 4"
		18/18	65.2	13' - 1"	14' - 1"	13' - 4"
		18/16	65.7	13' - 4"	14' - 4"	13' - 4"
		16/20	65.4	13' - 9"	15' - 8"	13' - 4"
		16/18	65.8	13' - 6"	15' - 4"	13' - 4"
		16/16	66.4	13' - 10"	15' - 10"	13' - 4"
7½ 2 Hour Fire Rating	1.852	20/20	76.1	9' - 10"	11' - 3"	10' - 8"
		20/18	76.6	9' - 10"	11' - 7"	10' - 8"
		20/16	77.1	10' - 1"	11' - 10"	10' - 11"
		18/20	76.8	12' - 4"	12' - 11"	13' - 7"
		18/18	77.3	12' - 7"	13' - 2"	13' - 8"
		18/16	77.8	12' - 9"	13' - 6"	13' - 4"
		16/20	77.5	13' - 2"	14' - 8"	13' - 4"
		16/18	77.9	12' - 11"	14' - 5"	13' - 4"
		16/16	78.4	13' - 4"	14' - 10"	13' - 4"
8¾ 3 Hour Fire Rating	2.083	20/20	85.2	9' - 5"	10' - 10"	10' - 2"
		20/18	85.7	9' - 5"	11' - 1"	10' - 3"
		20/16	86.2	9' - 8"	11' - 4"	10' - 6"
		18/20	85.8	12' - 1"	12' - 4"	13' - 0"
		18/18	86.3	12' - 3"	12' - 8"	13' - 1"
		18/16	86.8	12' - 5"	12' - 11"	13' - 1"
		16/20	86.5	12' - 10"	14' - 1"	13' - 4"
		16/18	87.0	12' - 7"	13' - 10"	13' - 4"
		16/16	87.5	13' - 0"	14' - 3"	13' - 4"

Shaded area the span is governed by maximum deck panel length of 40 feet; 20 feet for double span, 13 feet 4 inches for triple span.

3WxH-36 Composite Deck 2.4

5" Total Slab Depth

Normal Weight Concrete (145 pcf)

Concrete Volume 1.080yd³/100ft²

3WxH-36 5" Slab Depth, 145 pcf NWC

Gage	Gage	Single	Double	Triple		Gage	Single	Double	Triple
	22	10' - 1"	11' - 0"	11' - 4"		19	12' - 3"	13' - 10"	14' - 4"
	21	11' - 0"	11' - 9"	12' - 2"		18	12' - 7"	15' - 2"	14' - 9"
	20	11' - 9"	12' - 6"	12' - 11"		16	13' - 3"	16' - 7"	15' - 7"

Gage	Vertical Load Span (ft-in)	8'-0"	8'-6"	9'-0"	9'-6"	10'-0"	10'-6"	11'-0"	11'-6"	12'-0"	12'-6"	13'-0"	13'-6"	14'-0"	14'-6"	15'-0"
19	ASD & LRFD - Available Superimposed Load Capacity, W (psf)															
	ASD, W/Ω	469	412	364	323	288	258	232	210	190	172	157	143	131	119	109
	LRFD, ϕW	630	552	487	431	384	343	308	277	250	226	205	186	170	154	141
	L/360	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	LRFD - Available Diaphragm Shear Capacity, ϕS _n (plf / ft) 36/4 Attachment Pattern															
	Arc Spot Weld 1/2" Effective Dia	2579	2540	2506	2497	2468	2442	2418	2397	2377	2376	2358	2342	2327	2313	2300
	PAF Base Steel ≥ .25"	2295	2273	2254	2258	2241	2226	2212	2200	2188	2194	2184	2174	2165	2156	2149
	PAF Base Steel ≥ 0.125"	2274	2253	2235	2241	2225	2210	2197	2185	2174	2181	2171	2161	2153	2145	2137
	#12 Screw Base Steel ≥ .0385"	2258	2238	2220	2227	2212	2198	2185	2174	2163	2170	2161	2152	2144	2136	2129
	Concrete + Deck =	44.8	psf			I _{cr} = 50.6	in ⁴ /ft	ASD	M _{no} /Ω=	42.9	kip-in/ft		V _n /Ω =	3.82	kip/ft	
(I _{cr} +I _u)/2 =	81.72	in ⁴ /ft			I _u = 112.8	in ⁴ /ft	LRFD	ϕM _{no} =	65.7	kip-in/ft		ϕ V _n =	5.73	kip/ft		

Gage	Vertical Load Span (ft-in)	8'-0"	8'-6"	9'-0"	9'-6"	10'-0"	10'-6"	11'-0"	11'-6"	12'-0"	12'-6"	13'-0"	13'-6"	14'-0"	14'-6"	15'-0"
18	ASD & LRFD - Available Superimposed Load Capacity, W (psf)															
	ASD, W/Ω	527	463	409	364	325	292	263	238	216	196	179	162	145	131	118
	LRFD, φW	709	622	549	487	434	389	349	315	285	258	235	214	195	178	163
	L/360	-	-	-	-	-	-	-	-	-	-	-	162	145	131	118
	LRFD - Available Diaphragm Shear Capacity, φS _n (plf / ft) 36/4 Attachment Pattern															
	Arc Spot Weld 1/2" Effective Dia	2679	2634	2594	2587	2554	2523	2496	2470	2447	2448	2427	2408	2391	2374	2359
	PAF Base Steel ≥ .25"	2352	2326	2303	2312	2292	2274	2258	2243	2229	2238	2226	2214	2204	2194	2184
	PAF Base Steel ≥ 0.125"	2328	2304	2282	2292	2273	2256	2240	2226	2213	2223	2211	2200	2190	2181	2172
	#12 Screw Base Steel ≥ .0385"	2311	2288	2267	2278	2259	2243	2228	2215	2202	2212	2201	2190	2181	2171	2163
	Concrete + Deck =	45.0	psf			I _{cr} = 55.4	in ⁴ /ft	ASD	M _{no} /Ω=	47.9	kip-in/ft		V _n /Ω =	3.82	kip/ft	
(I _{cr} +I _u)/2 =	85.38	in ⁴ /ft			I _u = 115.4	in ⁴ /ft	LRFD	φM _{no} =	73.2	kip-in/ft		φ V _n =	5.73	kip/ft		

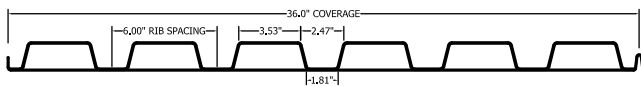
Age	Vertical Load Span (ft-in)	8'-0"	8'-6"	9'-0"	9'-6"	10'-0"	10'-6"	11'-0"	11'-6"	12'-0"	12'-6"	13'-0"	13'-6"	14'-0"	14'-6"	15'-0"
16	ASD & LRFD - Available Superimposed Load Capacity, W (psf)															
	ASD, W/Ω	649	571	505	450	403	362	326	285	251	222	197	176	158	142	129
	LRFD, φW	874	768	679	604	540	484	436	395	358	326	297	271	248	228	209
	L/360	-	-	-	-	-	-	326	285	251	222	197	176	158	142	129
	LRFD - Available Diaphragm Shear Capacity, φS _n (plf / ft)															
	36/4 Attachment Pattern															
	Arc Spot Weld 1/2" Effective Dia	2898	2840	2788	2786	2742	2702	2666	2633	2602	2609	2581	2556	2533	2511	2491
	PAF Base Steel ≥ .25"	2477	2443	2413	2431	2405	2381	2359	2339	2321	2339	2322	2307	2292	2279	2266
	PAF Base Steel ≥ 0.125"	2431	2400	2372	2393	2368	2346	2326	2308	2291	2310	2294	2279	2266	2253	2242
	#12 Screw Base Steel ≥ .0385"	2431	2400	2373	2393	2369	2346	2326	2308	2291	2310	2294	2280	2266	2254	2242
Concrete + Deck =		45.7	psf		I _{cr} = 64.9	in ⁴ /ft	ASD	M _{no} /Ω=	58.3	kip-in/ft		V _n /Ω =	3.82	kip/ft		
(I _{cr} +I _u)/2 =		92.79	in ⁴ /ft		I _u = 120.7	in ⁴ /ft	LRFD	φM _{no} =	89.2	kip-in/ft		φ V _n =	5.73	kip/ft		

All Gages	LRFD - Available Diaphragm Shear Capacity, ϕS_n (plf / ft) for all vertical load spans, WWF Size or Area of Steel per foot width					
	3/4" Welded Shear Studs	6x6 W1.4xW1.4 $A_s = 0.028 \text{ in}^2/\text{ft}$	6x6 W2.9xW2.9 $A_s = 0.058 \text{ in}^2/\text{ft}$	6x6 W4.0xW4.0 $A_s = 0.080 \text{ in}^2/\text{ft}$	4x4 W4xW4 $A_s = 0.120 \text{ in}^2/\text{ft}$	4x4 W6xW6 $A_s = 0.180 \text{ in}^2/\text{ft}$
	12 in o.c.	3200	4550	5540	7340	10040
	24 in o.c.	3200	4550	5540	7340	7750
	36 in o.c.	3200	4550	5170	5170	5170

2.3 DGBF-36 & BF-36



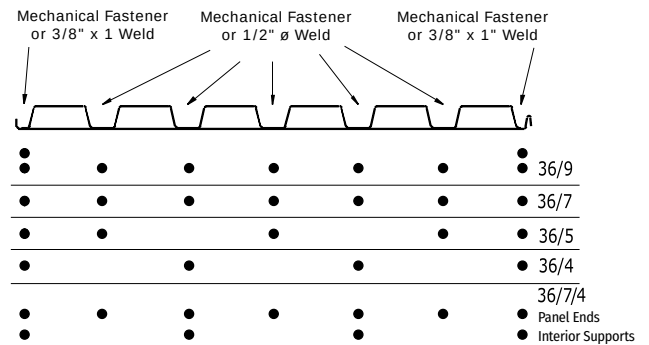
BF-36



Note: Weld sizes are effective not visible. Refer to AISI S100-2016 or AWS D1.3 for additional welding requirements.

*Die Set (Swedge) ends are no longer standard but can be performed upon request

Attachment Patterns



Panel Properties

Gauge	Weight	Base Metal Thickness	Yield Strength	Tensile Strength	Gross Section Properties				
					Area	Moment of Inertia	Distance to N.A. from Bottom	Top Section Modulus	Radius of Gyration
	w psf	t _{beam} in	F _y ksi	F _u ksi	A _g in ² /ft	I _g in ⁴ /ft	y _b in	S _g in ³ /ft	r in
20/20	3.69	0.0359 / 0.036	50	65	1.032	0.433	0.55	0.435	0.647
20/18	4.16	0.0359 / 0.047	50	65	1.171	0.470	0.50	0.444	0.633
20/16	4.68	0.0359 / 0.059	50	65	1.321	0.500	0.46	0.451	0.616
18/20	4.35	0.0478 / 0.036	50	65	1.218	0.527	0.61	0.562	0.657
18/18	4.83	0.0478 / 0.047	50	65	1.357	0.570	0.56	0.574	0.649
18/16	5.35	0.0478 / 0.059	50	65	1.508	0.613	0.52	0.584	0.638
16/20	5.03	0.0598 / 0.036	50	65	1.404	0.610	0.66	0.683	0.659
16/18	5.51	0.0598 / 0.047	50	65	1.542	0.663	0.61	0.698	0.656
16/16	6.03	0.0598 / 0.059	50	65	1.693	0.713	0.57	0.711	0.649

Gauge	Effective Section Properties at F _y for Bending Strength					Effective Section Properties at Service Load for Deflection			
	Area	Section Modulus	Distance to N.A. from Bottom	Section Modulus	Distance to N.A. from Bottom	Moment of Inertia	Moment of Inertia	Uniform Load Only	
								I _d = (2I _e +I _g)/3	
	A _e in ² /ft	S _e + in ³ /ft	y _b in	S _e - in ³ /ft	y _b in	I _e + in ⁴ /ft	I _e - in ⁴ /ft	I+ in ⁴ /ft	I- in ⁴ /ft
20/20	0.912	0.270	0.42	0.404	0.77	0.313	0.387	0.353	0.402
20/18	1.050	0.276	0.38	0.428	0.63	0.417	0.437	0.434	0.448
20/16	1.201	0.280	0.35	0.445	0.51	0.443	0.490	0.462	0.493
18/20	1.100	0.404	0.52	0.487	0.80	0.503	0.443	0.511	0.471
18/18	1.239	0.412	0.47	0.546	0.71	0.547	0.520	0.554	0.537
18/16	1.389	0.420	0.43	0.573	0.59	0.587	0.590	0.596	0.598
16/20	1.305	0.555	0.59	0.554	0.83	0.607	0.513	0.608	0.546
16/18	1.443	0.566	0.55	0.658	0.76	0.657	0.600	0.659	0.621
16/16	1.594	0.577	0.51	0.693	0.65	0.707	0.677	0.709	0.689

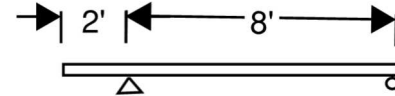
Reactions at Supports (plf) Based on Web Crippling

Gauge	Condition	Bearing Length of Webs							
		ASD (P _n /Ω) (lbs/ft width)				LRFD (ϕP _n) (lbs/ft width)			
		1"	1.5"	2"	3"	1"	1.5"	2"	3"
22	End	755	855	939	1081	1155	1308	1437	1654
	Interior	1184	1315	1427	1613	1760	1957	2122	2399
21	End	906	1024	1124	1290	1386	1567	1719	1974
	Interior	1429	1585	1715	1935	2126	2357	2552	2878
20	End	1059	1195	1309	1501	1621	1828	2003	2296
	Interior	1679	1857	2007	2260	2497	2762	2986	3361
19	End	1417	1592	1739	1987	2167	2435	2661	3039
	Interior	2264	2495	2689	3015	3367	3711	4000	4485
18	End	1800	2016	2197	2503	2754	3084	3362	3829
	Interior	2896	3181	3421	3823	4308	4731	5088	5687
16	End	2724	3033	3294	3732	4167	4641	5040	5710
	Interior	4434	4842	5187	5765	6595	7203	7715	8575
Constants		h = 1.388"			r = 0.156"	θ = 76.6°			

Code Reference: Seattle Building Code 2018

Deck Geometry

Corrugated deck span from wall to post support	8	ft
Cantilever span out of post	2	ft
Corrugated deck light weight concrete thickness	5	in
Deck support beam span	12	ft
Deck support beam size	W8X17	
Beam support post size	Ø5"	
Anchor support		



Design Load

Dead Load

Concrete deck = 42 in ² x 115 pcf=	33.54	plf
Corrugated steel=3.5 pcf=	3.5	plf
Base plate cover= Gauge 22 steel sheet=	1.22	plf
Total dead load=	38.3	plf
Handrail=	22.4	plf

Live Load

Pedestrian on deck=	100	plf
Pedestrian on handrail, larger of 50 plf or 250 lbs.	250	lbs

Snow Load

Snow load=	25	plf
------------	----	-----

Wind and snow load effects are not required for deck design

Load Combinations

1.2D+1.6L+0.5S=	218.4	plf	Controls
1.2D+1.6S=	85.9	plf	
Deck design moment=	1747.3	p-ft/ft	
Deck design shear=	878.7	plf	

Member Capacities

Corrugated deck

Uniform load capacity at L/360 deflection=	645.0	plf	Manufacture data
Uniform load demand=	218.4	plf	
	Okay		

Deck support beam

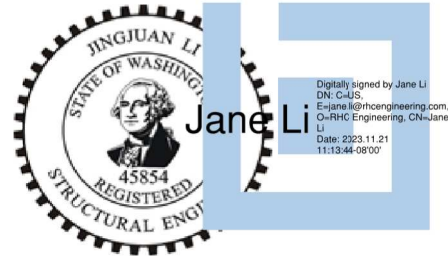
Maximum total uniform load based on AISC Manual	31400	lbs
Span L=	13	ft
Uniform load capacity=	2415.4	plf
Uniform load demand=	878.7	plf
	Okay	

Beam support post

Axial compression capacity=	20900	lbs	AISC Table 4-5, HSS5, Conservatively assumed with 20 feet height
Axial compression demand=	11423	lbs	
	Okay		

Wall anchor

Ø3/4" A307 Bolt		
Fy=	36	ksi
Abolt=	0.44	in ²
Tension capacity=	15840	lbs
Shear capacity=	6019.2	lbs
Bolt spacing=	3	ft
Anchor shear capacity=	2006.4	plf
Anchor shear demand=	878.7	plf
	Okay	



Handrail and Anchorage

1/4" fillet weld at 2" long

Beffw=

Lw1=

Lw2=

$I_w = \text{Beffw}^3 \cdot L_w / 12 + \text{Beffw} \cdot L_w^2 \cdot (L_w1/2)^2$

$S_w = I_w / (L_w1/2) =$

$F_w = 0.6 \cdot 50 =$

$M_{nw} = S_w \cdot F_w =$

Number of vertical rail=

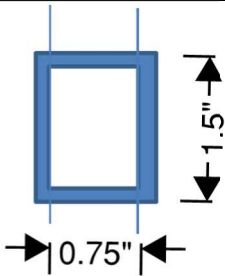
Rail base welding moment capacity, $\Phi M_{nwt} =$

Rail base welding moment demand, $M_{uwt} = 250 \cdot 3.5$

Rail base welding moment capacity, $\phi V_{nwt} =$

Rail base welding shear demand, $M_{uvt} = 250$

0.35	in	Effective fillet width
1.5	in	vertical fillet
0.75	in	Horizontal fillet
0.25	in^4	
0.33	in^3	
30.00	ksi	
829	p-ft/rail	
2.7	/ft	
1767.5	p-ft/ft	
875.0	p-ft/ft	
Okay		
33936	lbs	
250	plf	Conservatively assume 250 lbs load on each foot rail
Okay		



PLW3™-36/W3-36 FORMLOK® COMPOSITE DECKS

GRADE 50 STEEL

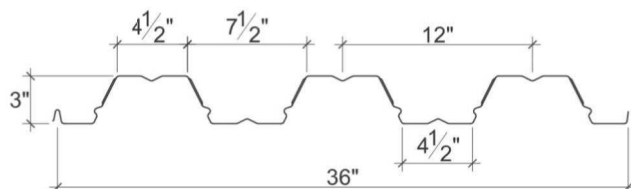
LRFD

W3 FORMLOK DECKS

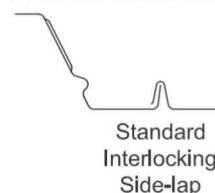
- PLW3-36 FormLok Deck used with PunchLok® II System
- W3-36 FormLok Deck used with TSWs or BPs
- W3-36-SS FormLok Deck used with Side-lap Screws



Nominal Dimensions



PLW3-36 or W3-36 FormLok



W3-36-SS FormLok



Section Properties

Deck Gage	Deck Weight w_{dd} (psf)	Base Metal Thickness t (in.)	Yield Strength F_y (ksi)	Effective Moment of Inertia at Service Load $I_d = (2I_e + I_g)/3$		Effective Section Modulus at $F_y = 50$ ksi		Vertical Web Shear ϕV_n (lb/ft)
				I_{d+} (in ⁴ /ft)	I_{d-} (in ⁴ /ft)	S_{e+} (in ³ /ft)	S_{e-} (in ³ /ft)	
22	1.9	0.0299	50	0.736	0.730	0.393	0.410	2074
20	2.3	0.0359	50	0.907	0.899	0.510	0.528	3587
18	2.9	0.0478	50	1.213	1.211	0.752	0.768	6515
16	3.5	0.0598	50	1.516	1.516	0.968	0.966	9422

Design Reactions at Supports Based on Web Crippling, ϕR_n (lb/ft)

Deck Gage	Bearing Length of Webs											
	One-Flange Loading						Two-Flange Loading					
	End Bearing				Interior Bearing		End Bearing				Interior Bearing	
	1 1/2"	2"	3"	4"	4"	8"	1 1/2"	2"	3"	4"	4"	8"
22	533	586	675	749	1157	1351	503	542	607	662	1341	1581
20	754	827	947	1049	1622	2010	763	819	913	992	1914	2405
18	1293	1410	1606	1771	2744	3436	1435	1532	1696	1834	3315	4218
16	1966	2135	2419	2658	4134	5130	2321	2469	2718	2927	5066	6392

Standard Features

- ASTM A653 SS GR50 Min., with G60 or G90, white or gray primer bottom optional
- ASTM A1008 SS GR50 Min. with gray primer bottom
- Standard lengths – 6'-0" to 40'-0"
- IAPMO UES ER-2018 and UL Listed
- Tables conform to ANSI/SDI C-2017

Optional Features

- Inquire regarding cost and lead times for:
 - Short cuts < 6'-0"
 - Sheet Lengths > 40'-0"
 - Alternative metallic and painted finishes
- Factory Vent Tabs

PLW3™-36/W3-36 FORMLOK® DECK-SLABS

LIGHT WEIGHT CONCRETE (110 pcf)

LFRD

		Maximum Unshored Spans				Composite Deck-Slab Properties			
Slab Depth		Deck Gage	Maximum Unshored Construction Clear Span			Concrete + Deck (psf)	Deflection $I_d = (I_{cr} + I_u)/2$ (in ⁴ /ft)	Moment ϕM_{no} (kip-ft/ft)	Shear ϕV_{no} (kip/ft)
Total	Topping		1	2	3				
5"	2"	22	11'-2"	11'-9"	12'-2"	34.0	5.73	4.91	4.01
		20	12'-6"	13'-6"	13'-11"	34.4	6.14	5.76	5.37
		18	13'-5"	16'-4"	15'-8"	35.0	6.88	7.35	5.75
		16	14'-1"	17'-7"	16'-6"	35.6	7.56	8.88	5.75
5½"	2½"	22	10'-8"	11'-3"	11'-8"	38.6	7.49	5.42	4.31
		20	12'-2"	12'-11"	13'-4"	39.0	8.01	6.35	5.67
		18	13'-0"	15'-8"	15'-3"	39.6	8.95	8.11	6.55
		16	13'-9"	17'-1"	16'-1"	40.2	9.80	9.79	6.55
6¼"	3¼"	22	10'-0"	10'-6"	10'-11"	45.4	10.75	6.22	4.79
		20	11'-8"	12'-3"	12'-7"	45.8	11.48	7.31	6.15
		18	12'-6"	14'-9"	14'-8"	46.4	12.79	9.35	7.83
		16	13'-3"	16'-5"	15'-6"	47.0	13.99	11.30	7.83

Note:

- Maximum unshored spans do not consider web-crippling. Required bearing should be determined based on specific span conditions.

Superimposed Design Load, ϕW_n , / Deflection at L/360 (psf)

LWC (110 pcf), $f'_c = 3000$ psi

Total Slab Depth	Deck Gage	Span (ft-in.)							
		8'-0"	9'-0"	10'-0"	11'-0"	12'-0"	13'-0"	14'-0"	16'-0"
5"	22	573/489	444/343	352/250	284/188	232/145	191/114	159/91	112/61
	20	678/524	527/368	419/268	339/201	278/155	231/122	193/97	138/65
	18	877/587	684/412	546/300	444/225	366/174	306/136	258/109	187/73
	16	1066/645	833/453	667/330	544/248	450/191	377/150	319/120	234/80
5½"	22	630/639	488/448	387/327	311/245	254/189	210/148	174/119	122/79
	20	747/683	580/480	461/349	373/262	306/202	253/159	212/127	151/85
	18	966/763	753/536	601/391	488/293	403/226	336/177	283/142	206/95
	16	1175/836	918/587	734/428	599/321	495/247	415/195	351/156	257/104
6¼"	22	723/917	560/644	443/469	356/353	291/271	240/213	199/171	139/114
	20	858/979	666/688	529/501	427/376	350/290	290/228	243/182	173/122
	18	1112/1092	867/767	692/559	562/420	463/323	386/254	325/203	236/136
	16	1355/1194	1059/838	847/611	690/459	571/353	478/278	404/222	296/149

Notes:

- For high loads long term concrete creep should be considered.
- Use Composite Deck-Slab Strength Web Based Solutions for alternate slabs or ASD design.



ALTERATIONS TO THE TRIANGLE MARKET GROUP PIKE PLACE AT PINE STREET

FRED BASSETTI & CO.
ARCHITECTS

STRUCTURAL ENGINEERS

MAHAN & HOWE
624-4488

MECHANICAL ENGINEERS

BENJAMIN S. NOTKIN & ASSOCIATES
682-3611

ELECTRICAL ENGINEERS

BEVERLY TRAVIS & ASSOCIATES
622-1447

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- 3 DEMOLITION ELEVATIONS
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- LOW PARTITION WALL (SEE DETAIL 6/14)
- MASONRY
- EXISTING WORK
- METAL
- CONCRETE
- BATT INSULATION
- GYPSUM BOARD
- WOOD MEMBERS ROUGH OR FRAMING
- WOOD FINISH

George R. Rolfe
Executive Director
Urban Const. Co., Inc.
James Quinn, President

LEGAL DESCRIPTION

DESCRIPTION 6-5

THAT PORTION OF LOT 5 AND THE SOUTH HALF OF LOT 4, BLOCK "C", ADDITION TO THE TOWN OF SEATTLE, AS LAID OUT BY A.A. DENNY (COMMONLY KNOWN AS A.A. DENNY'S 4TH ADDITION TO THE CITY OF SEATTLE), ACCORDING TO PLAT RECORDED IN VOLUME 1 OF PLATS, PAGE 69, RECORDS OF KING COUNTY, WASHINGTON, LYING NORTHEASTERLY OF PIKE PLACE.

DESCRIPTION 6-6

THAT PORTION OF THE NORTH HALF OF LOT 4, BLOCK "C", ADDITION TO TOWN OF SEATTLE, AS LAID OUT BY A.A. DENNY (COMMONLY KNOWN AS A.A. DENNY'S 4TH ADDITION TO THE CITY OF SEATTLE), ACCORDING TO PLAT RECORDED IN VOLUME 1 OF PLATS, PAGE 69, IN KING COUNTY, WASHINGTON, LYING EAST OF PIKE PLACE, MORE PARTICULARLY DESCRIBED AS FOLLOWS: COMMENCING AT THE NORTHERLY CORNER OF SAID LOT, BEING THE COMMON CORNER OF LOTS 1 AND 4, AND AN ALLEY IN SAID BLOCK "C"; THENCE RUNNING SOUTHERLY ALONG SAID ALLEY 30 FEET; THENCE WESTERLY 22.45 FEET MORE OR LESS, TO THE EAST LINE OF THE STREET; THENCE NORTHWESTERLY ALONG THE EAST LINE OF THE STREET, 31.38 1/2 FEET TO THE INTERSECTION LINE BETWEEN LOTS 1 AND 5; THENCE EASTERLY 31.67 FEET ALONG SAID LINE TO THE POINT OF BEGINNING.

DESCRIPTION 6-7

THAT PORTION OF LOT 1, BLOCK "C", ADDITION TO THE TOWN OF SEATTLE AS LAID OUT BY A.A. DENNY (COMMONLY KNOWN AS A.A. DENNY'S 4TH ADDITION TO THE CITY OF SEATTLE), ACCORDING TO PLAT RECORDED IN VOLUME 1 OF PLATS, PAGE 69, RECORDS OF KING COUNTY, WASHINGTON, LYING NORTHEASTERLY OF PIKE PLACE.

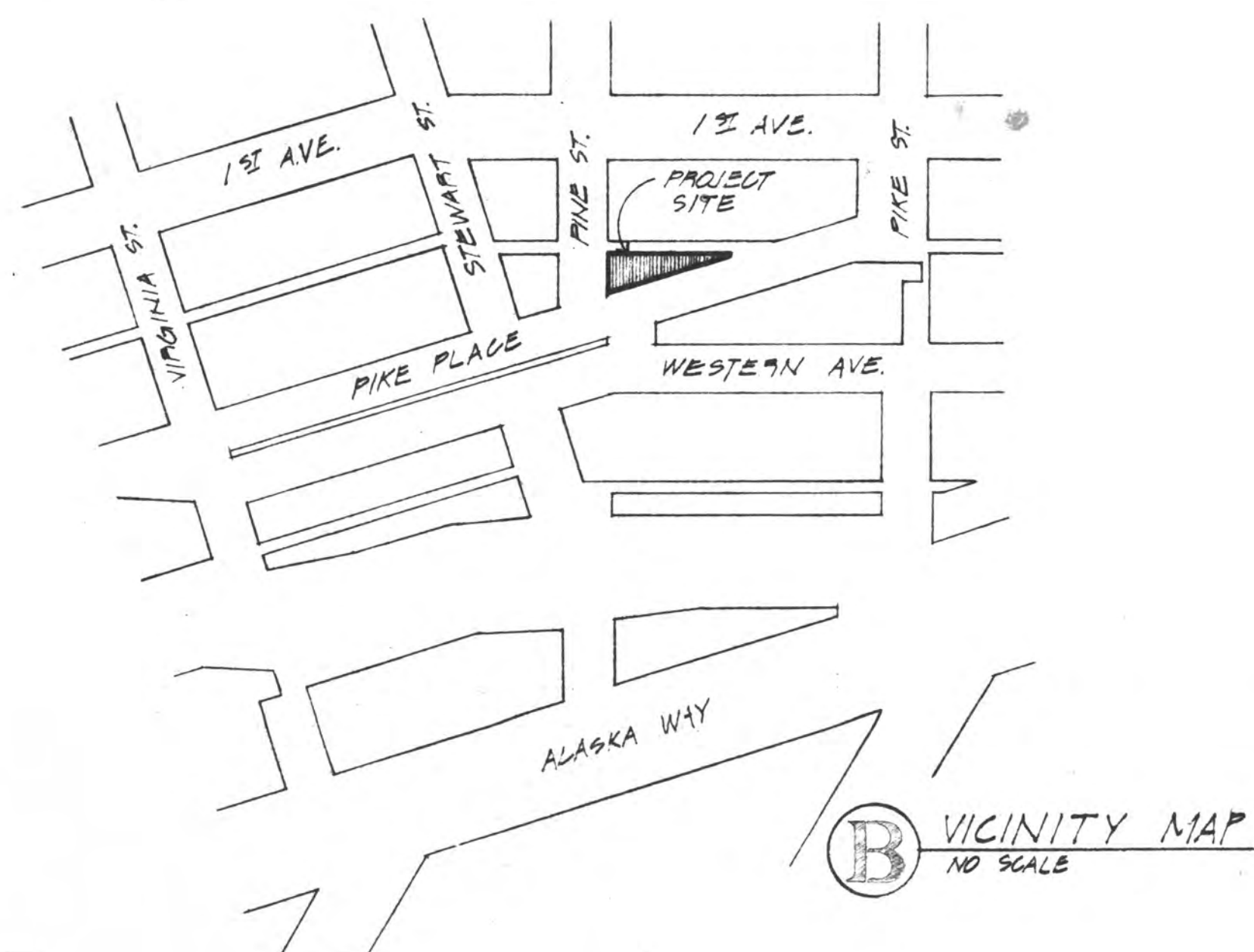
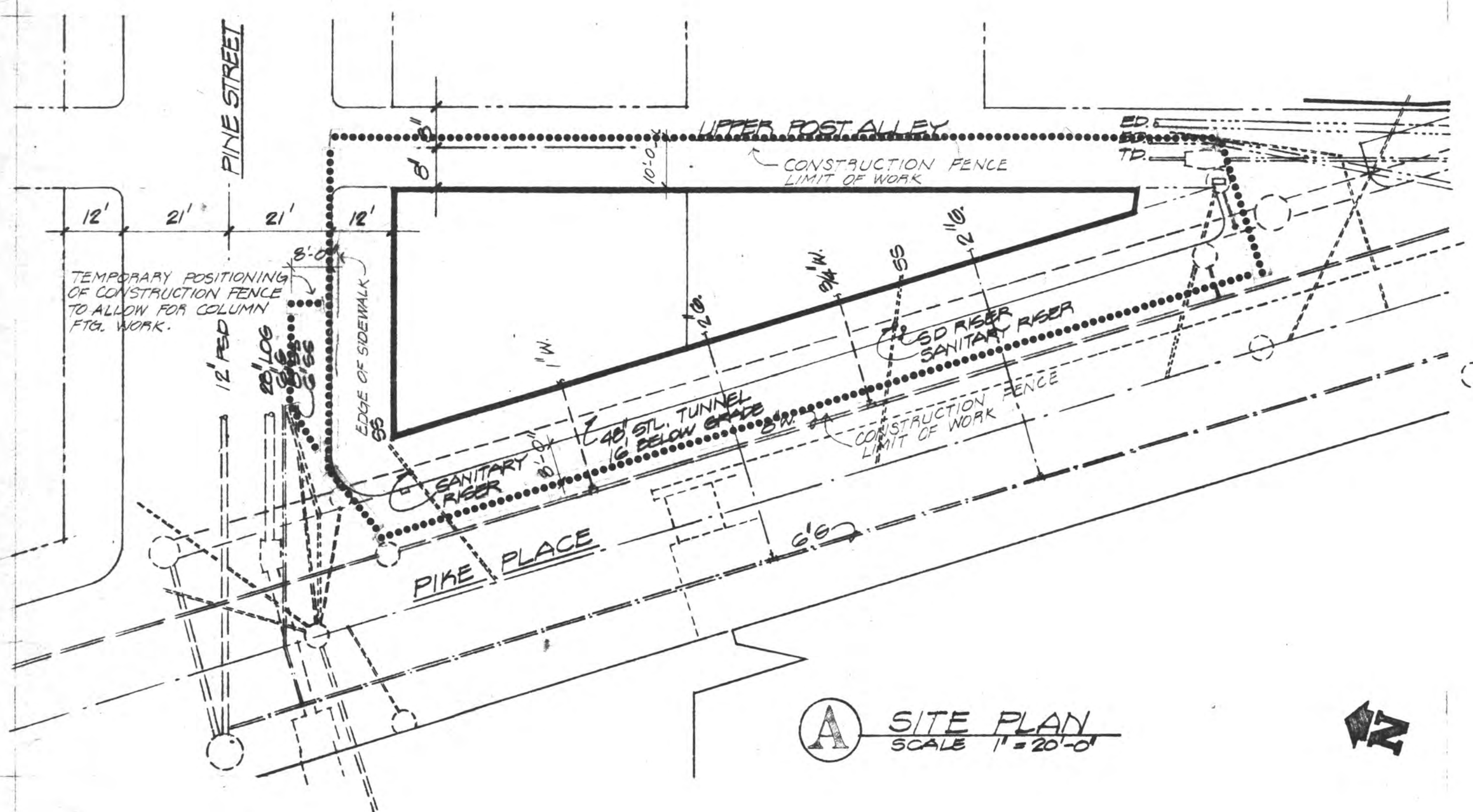
APPLICABLE CODE INFORMATION

LAND USE ZONE "CM" METRO COMMERCIAL ZONE
FIRE ZONE "1" ONE
OCCUPANCY TYPES "F-2" COMMERCIAL SHOPS
"H" APARTMENTS
CONSTRUCTION TYPE "III-1 HB."
OCCUPANCY SEPARATION "F-C" H = 1 HB.
EXISTING ALLEY SEPARATION 10 FT. (UNPROTECTED WINDOWS)
APPROX. SQ. FOOTAGE 2,499 S.F. SILVER GRUM
1,589 S.F. TRIANGLE
4,088 S.F. TOTAL

NOTE: DESIGN IN ACCORDANCE WITH THE SEATTLE BUILDING CODE AND ALL OTHER LOCAL CODES AND ORDINANCES.

SYMBOLS

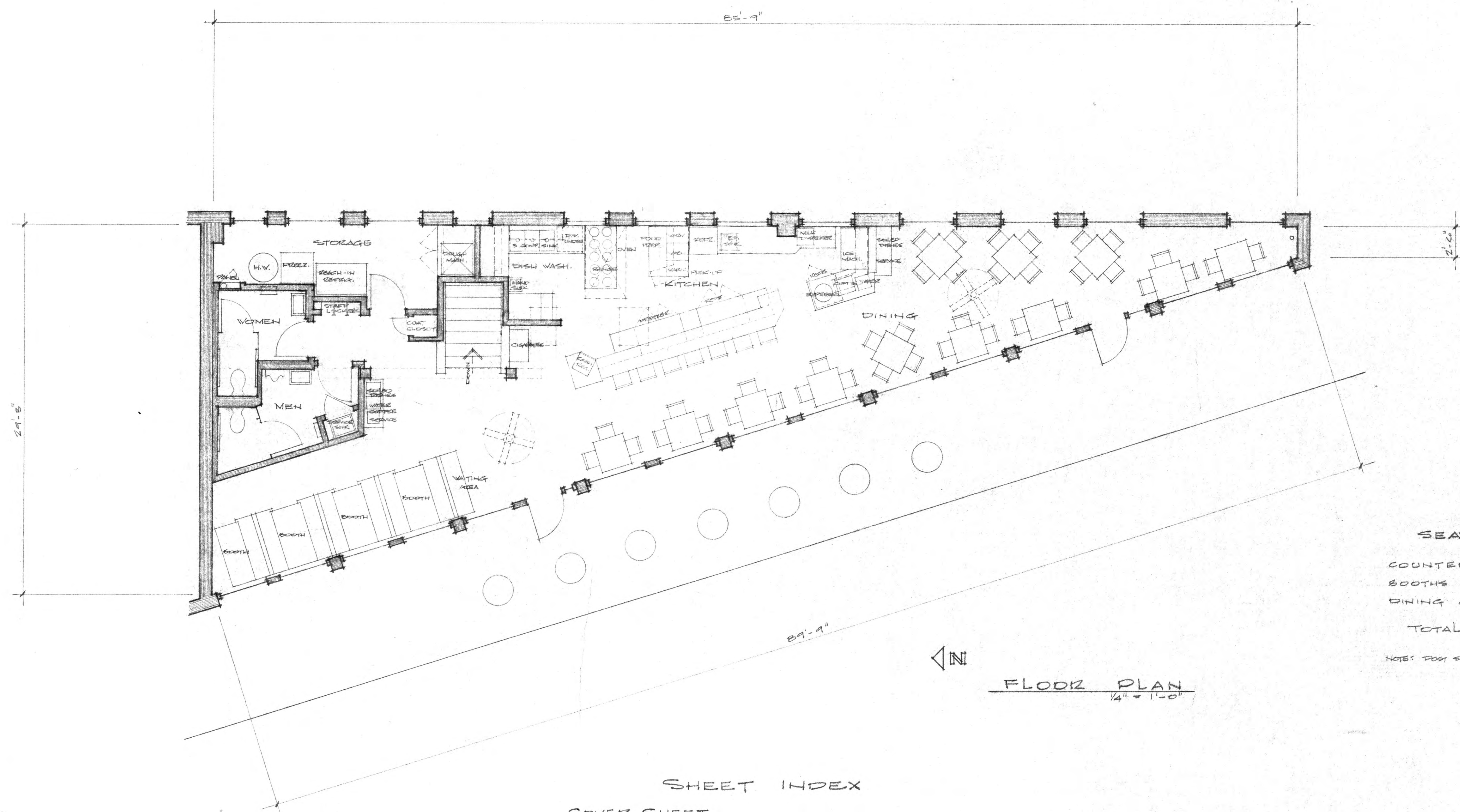
- B/A BUILDING SECTION
SECTION DESIGNATION
SHEET NUMBER
- D/A DETAIL CUT
DETAIL NUMBER
SHEET NUMBER
- E/E INTERIOR ELEVATION
ELEVATION NUMBER
SHEET NUMBER
- R/O ROOM NUMBER
- D/O DOOR NUMBER
- L/T LOUVER TYPE INDICATION
- C/G CHANGE OF FLOOR MATERIALS



FRED BASSETTI & COMPANY / ARCHITECTS
FRED BASSETTI
2027 - 5th AVE / SEATTLE WASHINGTON 98121 USA / NA 2-775
PHILIP NOTKIN

TRIANGLE MARKET GROUP
ALTERATIONS
PIKE PLACE AT PINE STREET

COVER SHEET
SITE PLAN / VICINITY MAP
LEGAL DESCRIPTION / SYMBOLS

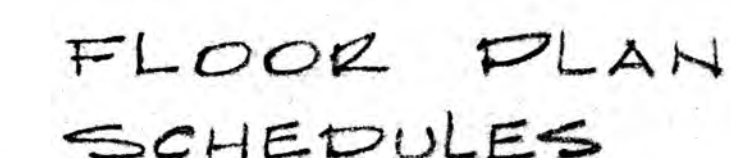


SHEET INDEX

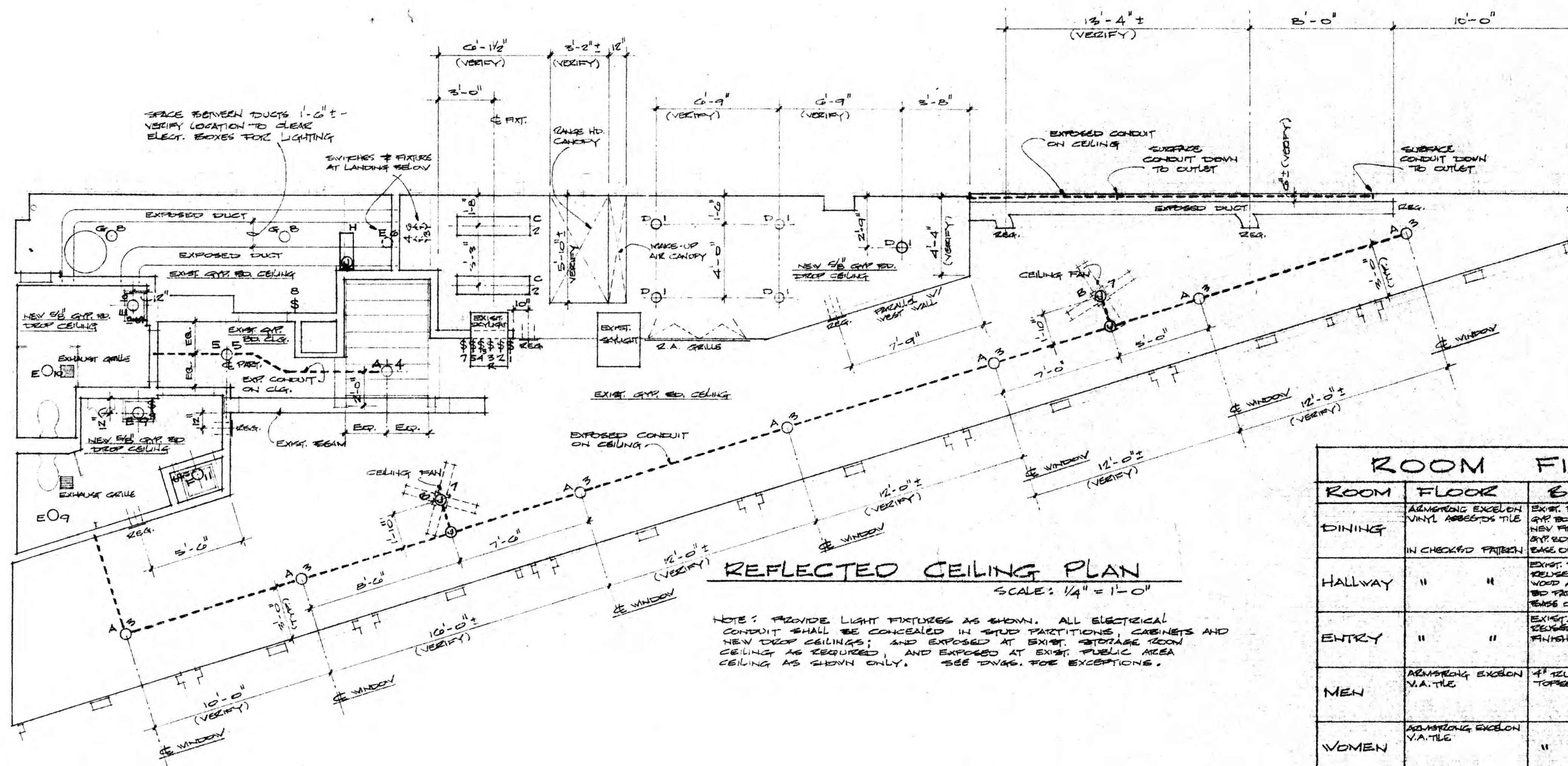
COVER SHEET	FURNISHINGS & SEATING LAYOUT
1	FLOOR PLAN SCHEDULES
2	REFLECTED CEILING PLAN & LIGHTING INTERIOR TOILET ELEVATIONS
3	KITCHEN AREA PLAN & ELEVATIONS DETAILS
4	HEATING & VENTILATION PLAN ELECTRICAL, PLUMBING & GAS SERVICE PLAN
5	SPECIFICATIONS EXTERIOR DETAILS MISCELLANEOUS

COPACABANA RESTAURANT
IN
THE TRIANGLE BUILDING
PIKE PLACE MARKET
SEATTLE, WASHINGTON
FURNISHINGS & SEATING LAYOUT
NOT FOR CONSTRUCTION

JOB NO. 1376 B-16-77



PLUMBING FIXTURE SCHEDULE				DOOR SCHEDULE								HARDWARE SCHEDULE	
SYMBOL	TYPE	DESCRIPTION	REMARKS	NO.	SIZE	TYPE	FRAME	FINISH	HCV.	REMARKS	GROUP	QTY.	DESCRIPTION
P-1	LAVATORY	"AMERICAN STANDARD" LEDGENERE E300.124 (16" x 14") w/ WALL HANGER & FAUCET POP-UP DRAIN, ENAMELED CAST IRON (WHITE) w/ SOAP DEPRESSION.		1	2'-0" x 3'-6" x 1 3/4"	FLUSH SOLID CORE WOOD - PAINT GRADE BIRCH	WOOD	SEE DETAIL B SH. 3	PAINT SEMIGLOSS ENAMEL	GROUP 1	12" x 20" STEEL DE. LOWER "ACQUITE" GSSF	1 1/2 PR.	BUTTS MCKINNEY A2714 10B 4 1/2 x 4 1/2 PRIVACY LATCH SARGENT 24-0605 RB 10B CLOSER SARGENT EB55-0 VALL STOP IVES 406 1/2 10B KICKPLATE CIRC 5024 B X B2 409 SIGN CIRC 409 X 1" WHITE LETTERS (1 MEN & 1 WOMEN)
P-2	SEWAGE SINK	"AMERICAN STANDARD" LACNELLE 799.1049 (22" x 18") w/ REM. GLASS w/ WALL HANGER & FAUCET. E340.242 w/ VACUUM BREAKER & TEAP 799B.170		2	2'-0" x 3'-6" x 1 3/4"	FLUSH SOLID CORE WOOD - PAINT GRADE BIRCH	WOOD	SEE DETAIL B SH. 3	PAINT SEMIGLOSS ENAMEL	GROUP 1	12" x 20" STEEL DE. LOWER "ACQUITE" GSSF	1 1/2 PR.	BUTTS MCKINNEY 2714 10B 4 1/2 x 4 1/2 LOCKSET SARGENT 24-0604 RB 10B FLOOR STOP IVES 436 10B
P-3	HAND SINK	"AMERICAN STANDARD" LEDGENERE E300.033 (18" x 12") w/ WALL HANGER & FAUCET POP-UP DRAIN, ENAMELED CAST IRON (WHITE) w/ SOAP DEPRESSION.		3	2'-0" x 3'-6" x 1 3/4"	FLUSH SOLID CORE WOOD - PAINT GRADE BIRCH	WOOD	SEE DETAIL B SH. 3	PAINT SEMIGLOSS ENAMEL	GROUP 2	12" x 20" STEEL DE. LOWER "ACQUITE" GSSF	1 1/2 PR.	BUTTS MCKINNEY 2714 10B 4 1/2 x 4 1/2 LOCKSET SARGENT 24-0604 RB 10B FLOOR STOP IVES 436 10B
U-1	URINAL	"AMERICAN STANDARD" VAPERBROOK G500.011 w/ TOP SPRID & WALL HANGER SLOW ROYAL FLUSH VALVE 100-11 & STRAKER 380240-32B		4	2'-4" x 3'-6" x 1 3/8"	FLUSH SOLID CORE WOOD - PAINT GRADE BIRCH	WOOD	SEE DETAIL B SH. 3	PAINT SEMIGLOSS ENAMEL	GROUP 3	12" x 20" STEEL DE. LOWER "ACQUITE" GSSF	1 1/2 PR.	BUTTS MCKINNEY 2714 10B 4 1/2 x 4 1/2 LOCKSET SARGENT 24-0604 RB 10B
V.C. 1	VATER CLOSET	"AMERICAN STANDARD" ARVAL 2477.010 w/ SEAT E320.114 (3500 C) MOLTEN & FLUSH VALVE SLOW ROYAL 110 - PROVIDE WALL HUNG SUPPORTS PER MANUF.	MOUNTING HEIGHT 18" PER HANDICAPPED	5	EXISTING 3'-0" x 6'-0"	EXISTING WOOD WITH GLASS	EXISTING	EXIST.	EXIST.	EXIST.	GROUP 3	1 1/2 PR.	BUTTS MCKINNEY 2714 10B 4 1/2 x 4 1/2 LOCKSET SARGENT 24-0604 RB 10B
	VEGETABLE SINK	"ELKAY" LR-1720 STAINLESS STEEL (20" x 14") OVERALL & 14" x 14" x 14" SINK	INSTALL INTO CABINET NOTE: PROVIDE INDIRECT DRAIN	6	EXISTING 3'-0" x 6'-0"	EXISTING WOOD WITH GLASS	EXISTING	EXIST.	EXIST.	EXIST.	GROUP 4	1 1/2 PR.	BUTTS MCKINNEY 2742 2 1/2" 10B (1 BY OWNER) LOCKER LOCK SARGENT 1054 10B (1 BY OWNER) PULL ROCKWOOD 147 1/2 X 6 10B (2 BY OWNER) MAGNETIC CATCH EP20 592
3	COMBINATION SINK	NEW STAINLESS STEEL AS SHOWN ON ENDS - 14 GA TYPE 304 - ROUNDED OR CHANNEL RIM DESIGN - 8" MIN. HT. SANITARY BACKPLASH 2" THICK w/ ANGLE TOP - DOUBLE SINKEL FAUCETS w/ LONG SPOUT REVERSED TEE HOSE CONNECTION & VACUUM BREAKER - PROVIDE PROVIDING EQUIP.	SINK COMPARTMENT MIN. 10" DEPTH NOTE: PROVIDE SHOP APPROVAL	7	EXISTING 3'-0" x 6'-0"	EXISTING WOOD WITH GLASS	EXISTING	EXIST.	EXIST.	EXIST.	GROUP 4	1 1/2 PR.	BUTTS MCKINNEY 2742 2 1/2" 10B (1 BY OWNER) LOCKER LOCK SARGENT 1054 10B (1 BY OWNER) PULL ROCKWOOD 147 1/2 X 6 10B (2 BY OWNER) MAGNETIC CATCH EP20 592
	DISHWASHER	"HOBART" UNDERCOUNTER MODEL WM-C1 w/ CHLORINIZER (24 1/2" HIGH x 24" x 24")		8	1'-0" x 3'-6" x 1 3/4"	PAINT GRADE BIRCH VENEER CORE w/ FINISHED EDGE	WOOD	SEE DETAIL 9 SH. 3	PAINT SEMIGLOSS ENAMEL	GROUP 4	1 1/2 PR.	NOTE: CATALOG NOS. COORDINATED WITH CONTRACT HARDWARE INC., PHONE: 682-4470.	



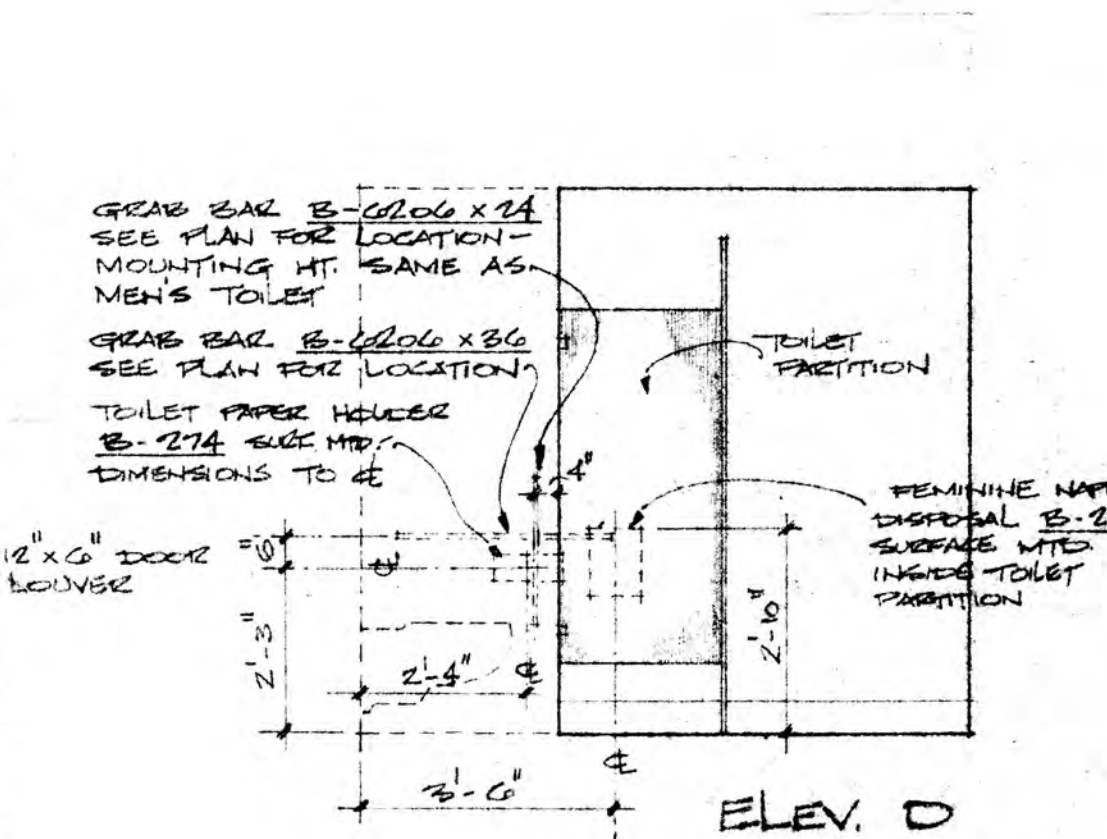
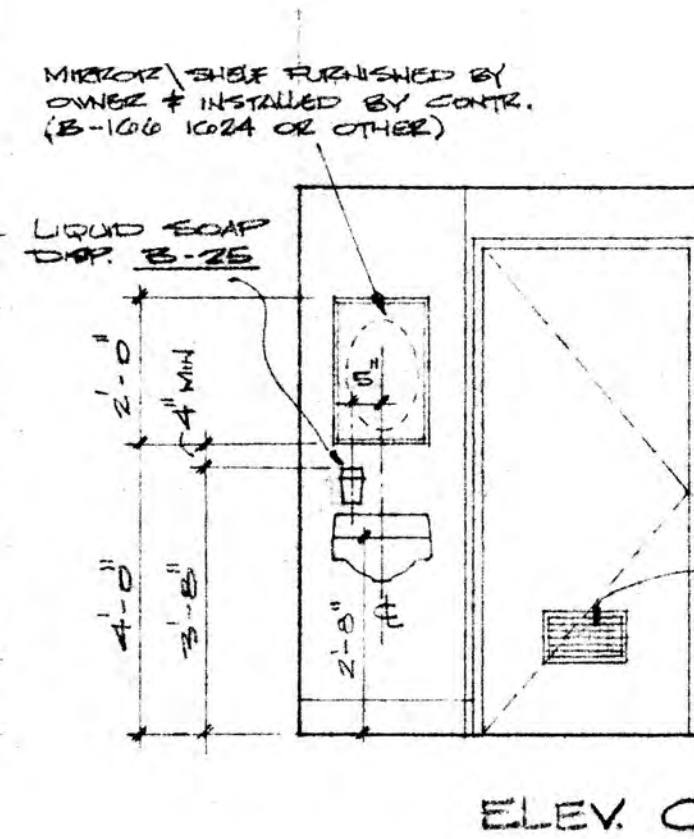
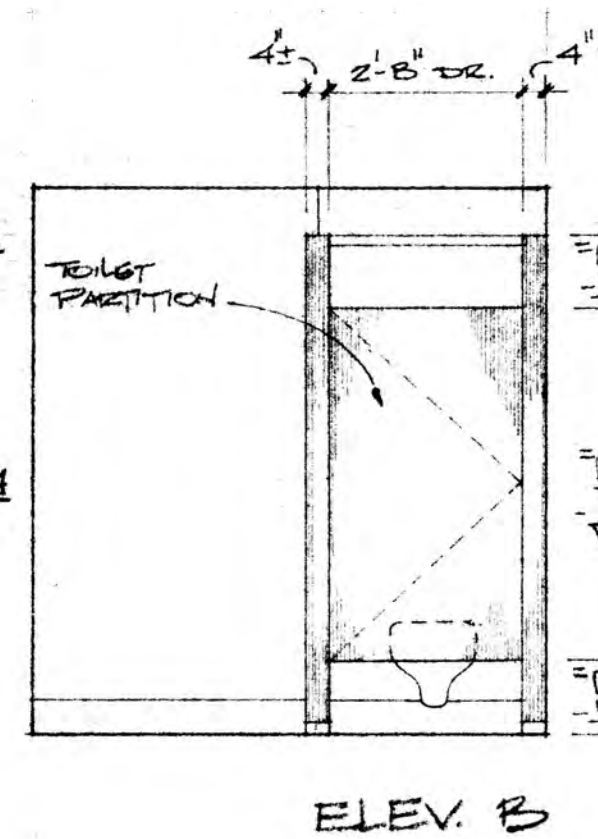
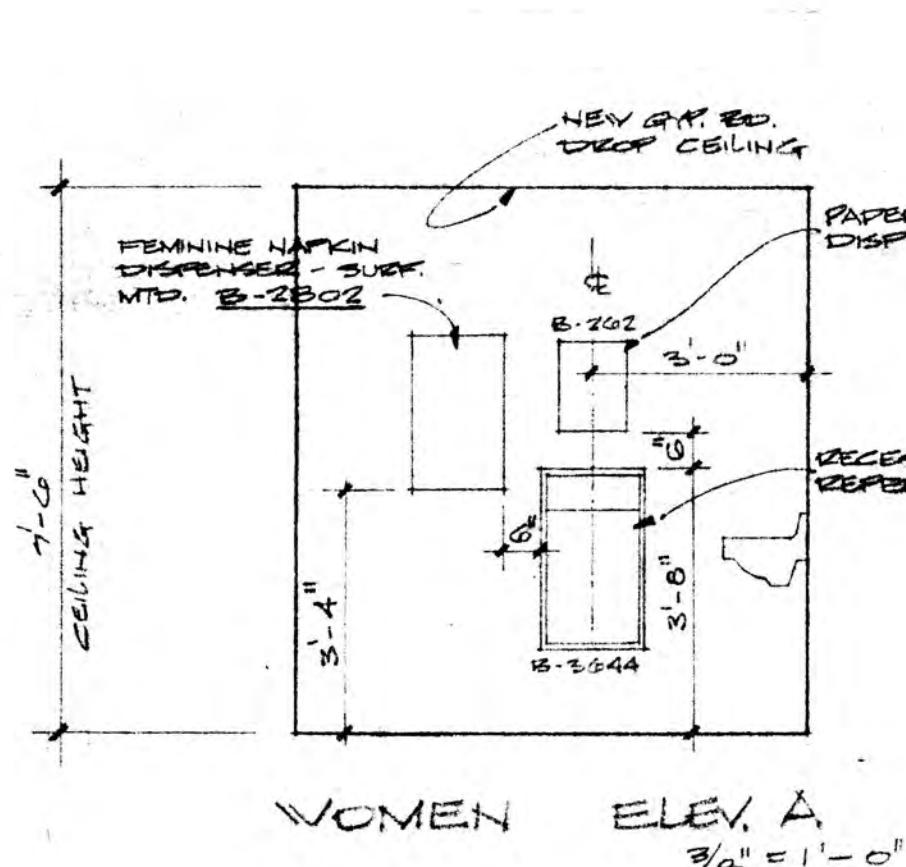
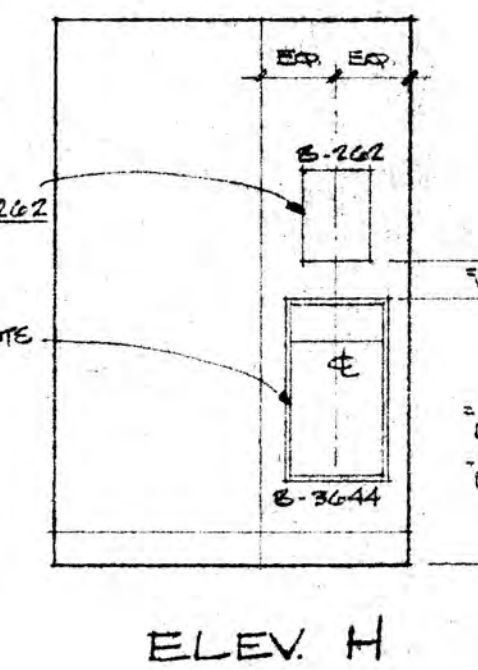
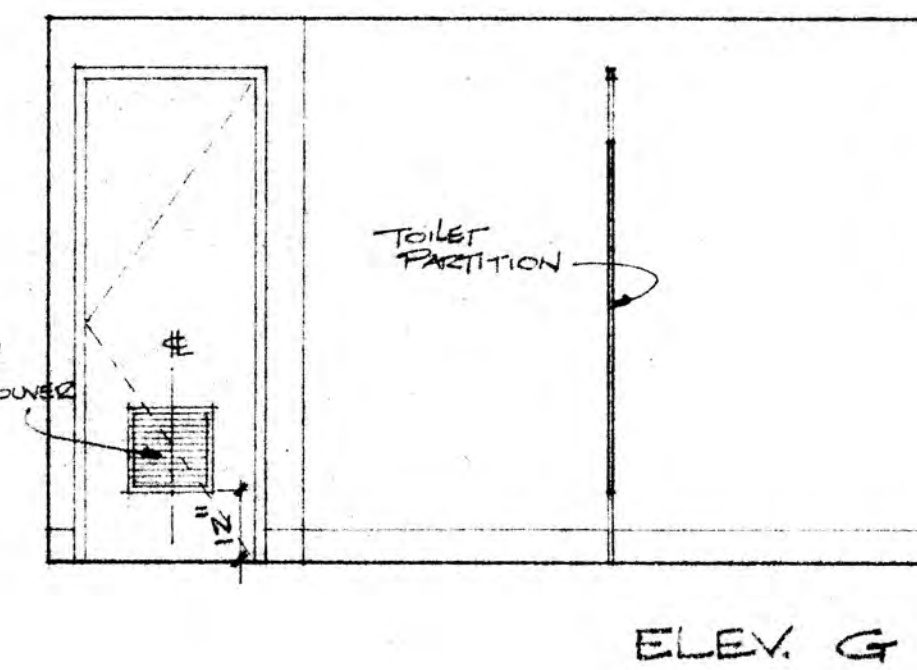
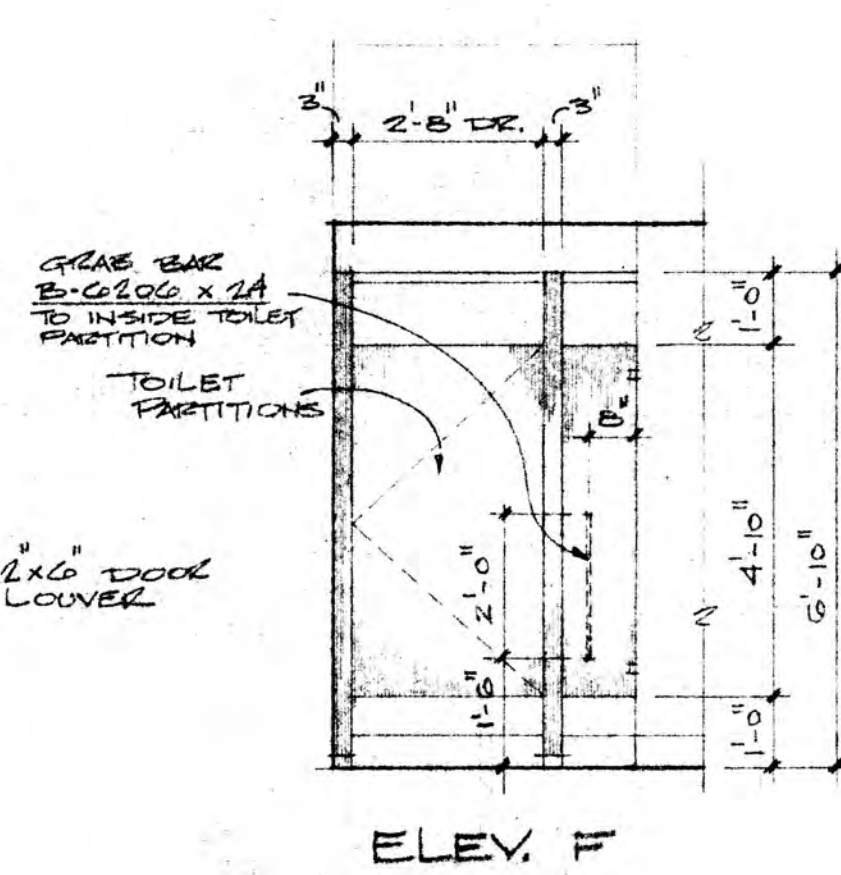
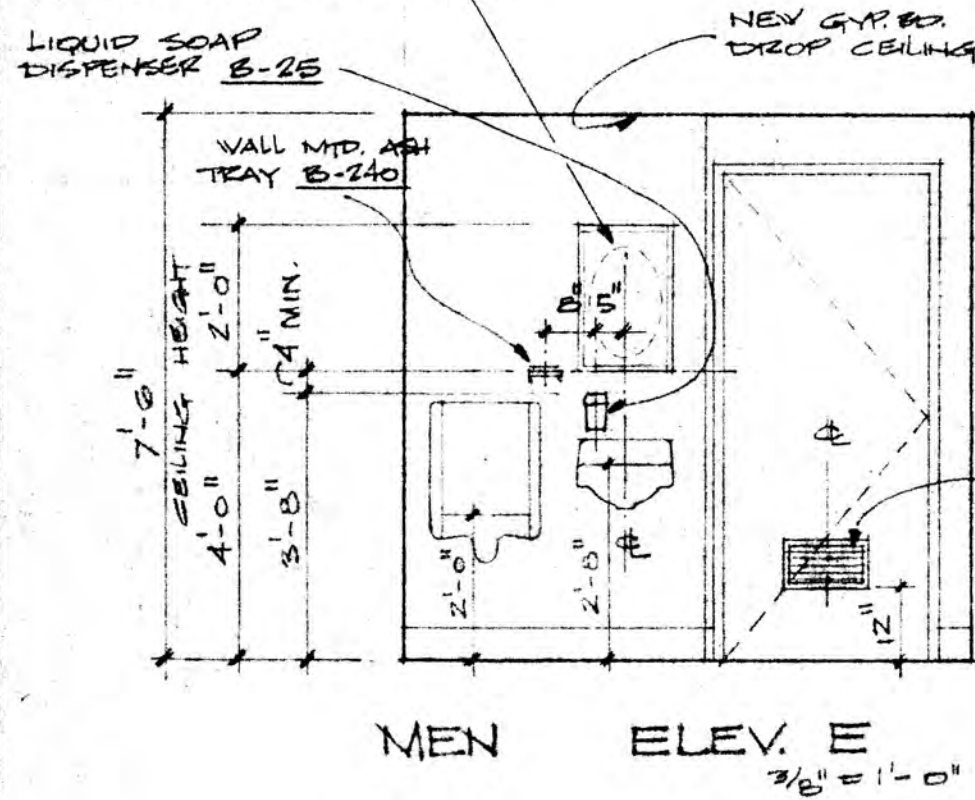
LIGHT & FAN FIXTURE SCHEDULE

- A ZEESE CHAIN PENDANT W/ GLASS GLOBE FURNISHED BY OWNER - REMOVE, SHORTEN CHAIN AND INSTALL AS DIRECTED BY ARCHITECT. MAX. 100W INCANDESCENT.
- B CEILING FAN - "HUNTER" 52" NO. 22272 CHESTNUT BROWN 115V. (MAX 7000 CFM, 200 RPM, 145W) W/ VARIABLE SPEED WALL SWITCH CONTROL (VERIFY) - PROVIDE BLOCKING BETWEEN CLG JOISTS CONCEALED TO SUPPORT 50 LB. WEIGHT.
- C 4 FLUORESCENT - 2 TUBE 40 W - COLUMBIA MODEL-10 NO. FT-UC-240-MIG SURFACE MTD. W/ FULLY CLOSED TOP REFLECTOR.
- D INCANDESCENT 72-00V CEILING SURFACE MTD. - "LIGHTSCAPE" 011-30-70 W/ SATIN WHITE GLASS (11"x6") & POLISHED BRASS FRAME.
- E INCANDESCENT 2-00V CEILING SURFACE MTD. - "LIGHTSCAPE" 009-30-70 W/ SATIN WHITE GLASS (9"x5") & POLISHED BRASS FRAME.
- F INCANDESCENT 1-00V CEILING SURFACE MTD. - "LIGHTSCAPE" 007-30-70 W/ SATIN WHITE GLASS (7"x4") & POLISHED BRASS FRAME.
- G INCANDESCENT REFLECTOR 1-100W W/ 1/2" CONDUIT STEEL - "ASCITE" 100-100 (12" DIA X 8" HT) - MOUNTING HEIGHT AS DIRECTED BY ARCHITECT.
- H 2' FLUORESCENT COMPACT - 1 TUBE 20 W - "STERLING" 324-1 W/ SWITCH & OUTLET.

ROOM FINISH SCHEDULE

ROOM	FLOOR	BASE	WALLS	CEILING	REMARKS
DINING	ARMSTRONG EXCELON VINYL ASBESTOS TILE	EXIST. THE WOOD AT GYP. BO. CEUSED & NEW THE WOOD AT NEW GYP. BO. FRT. ALL WOOD BASE OIL FIN. NATURAL	EXIST. BRICK NO CHANGE - EXIST. PLASTER & NEW GYP. BO. FINISH & PAINT	EXISTING GYPSUM BOARD FINISH & PAINT	NO BASE AT EXIST. FLOOR TILE 1/2" X 1/2" (TYPICAL) FINISH & PAINT EXIST. WOOD POST & BEAM, 2 COATS OIL FIN. BASE (TYPICAL)
HALLWAY	"	"	"	"	"
ENTRY	"	"	"	"	"
MEN	ARMSTRONG EXCELON V.A. TILE	4" FLEXIBLE CONE TOPSEET	GYPSUM BOARD FINISH & EPOXY PAINT	GYPSUM BOARD FINISH & EPOXY PAINT	COME INTERIOR WOOD STAIRS W/ "FLEXA" FLEXIBLE RUBBER STAIR TREAD TACK 110 ROUND W/ SAFETY RIB & RUBBER TIRE BASE - COLORS SELECTED BY ARCHITECT
WOMEN	ARMSTRONG EXCELON V.A. TILE	"	"	"	"
KITCHEN/ DISHWASH	ARMSTRONG EXCELON V.A. TILE	4" FLEXIBLE CONE TOPSEET	GYPSUM BOARD FINISH & PAINT, SEMI-GLOSS ENAMEL	GYPSUM BOARD FINISH & PAINT, SEMI-GLOSS ENAMEL	"
STORAGE	"	"	"	"	"

MIRROR FURNISHED BY OWNER & INSTALLED BY OWNER. (B-106 1024 OR OTHER)



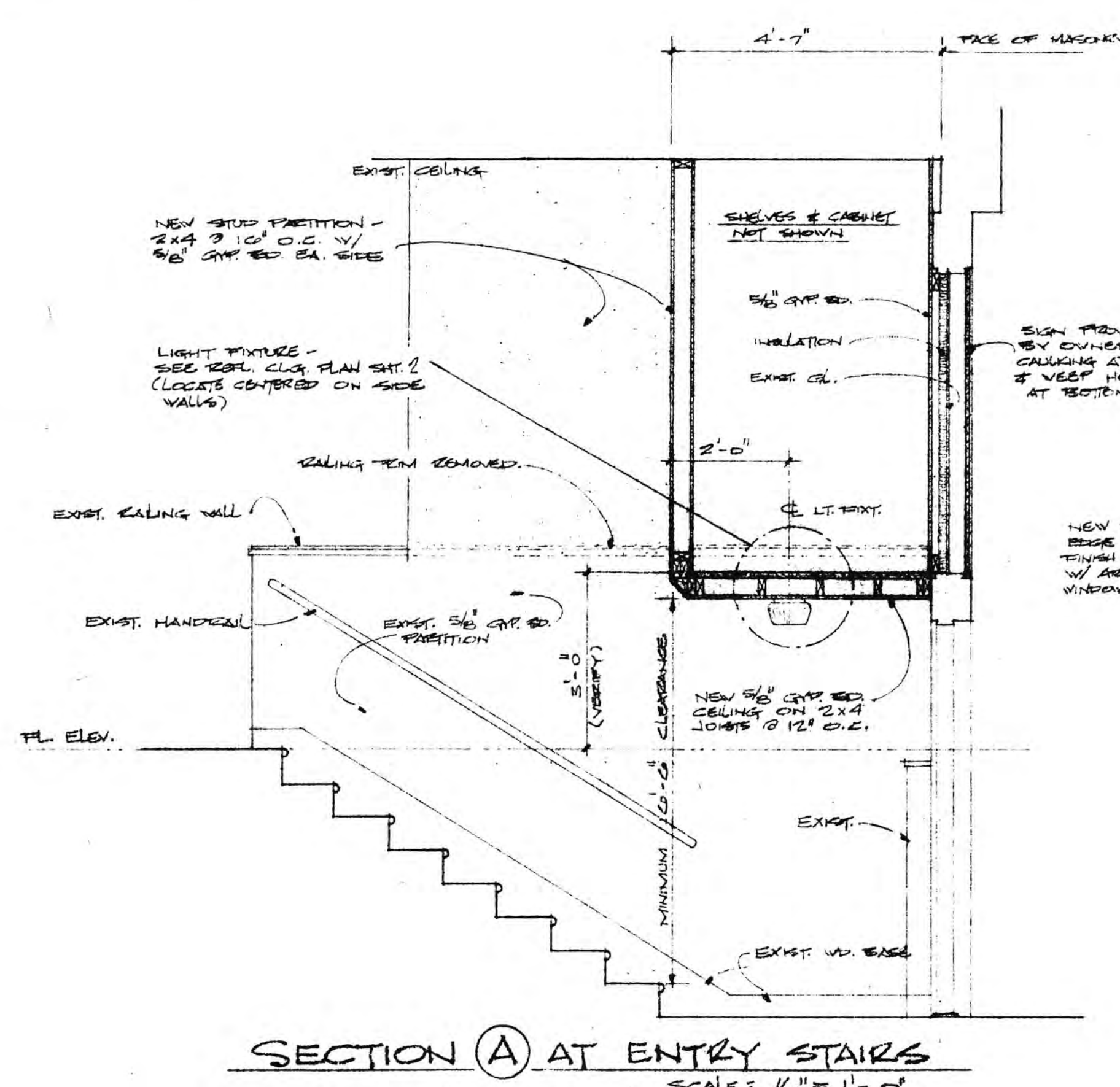
REFLECTED CEILING PLAN & LIGHTING
INTERIOR TOILET ELEVATIONS

COPACABANA RESTAURANT
TRIANGLE BLDG. IN THE PIKE PLACE MARKET
SEATTLE, WASHINGTON
1518 PIKE PLACE

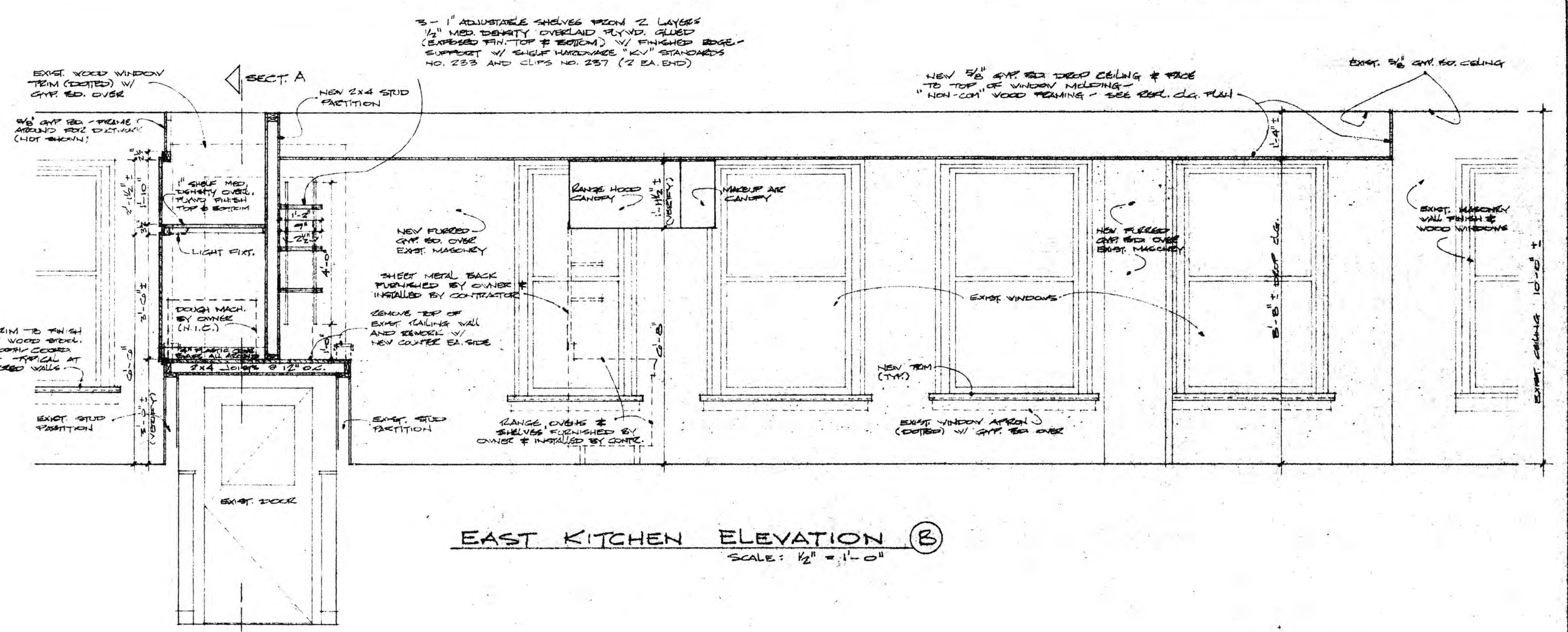
REGISTERED
ARCHITECT
CARL B. HELLER
STATE OF WASHINGTON

ARCHITECT - URBAN CONSULTANT
CARL B. HELLER
SEATTLE, WASH. 98101
PH 933-7818

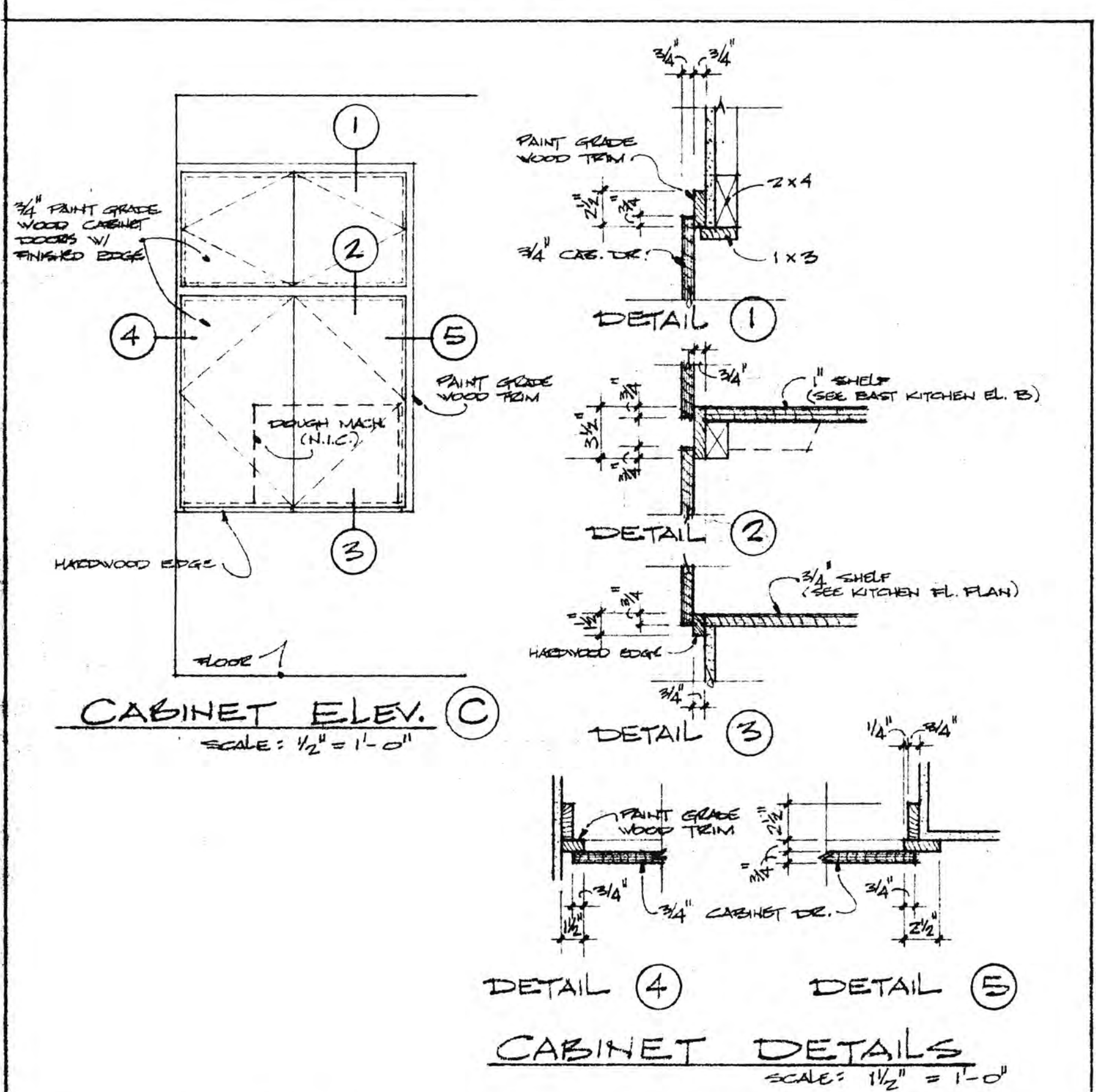
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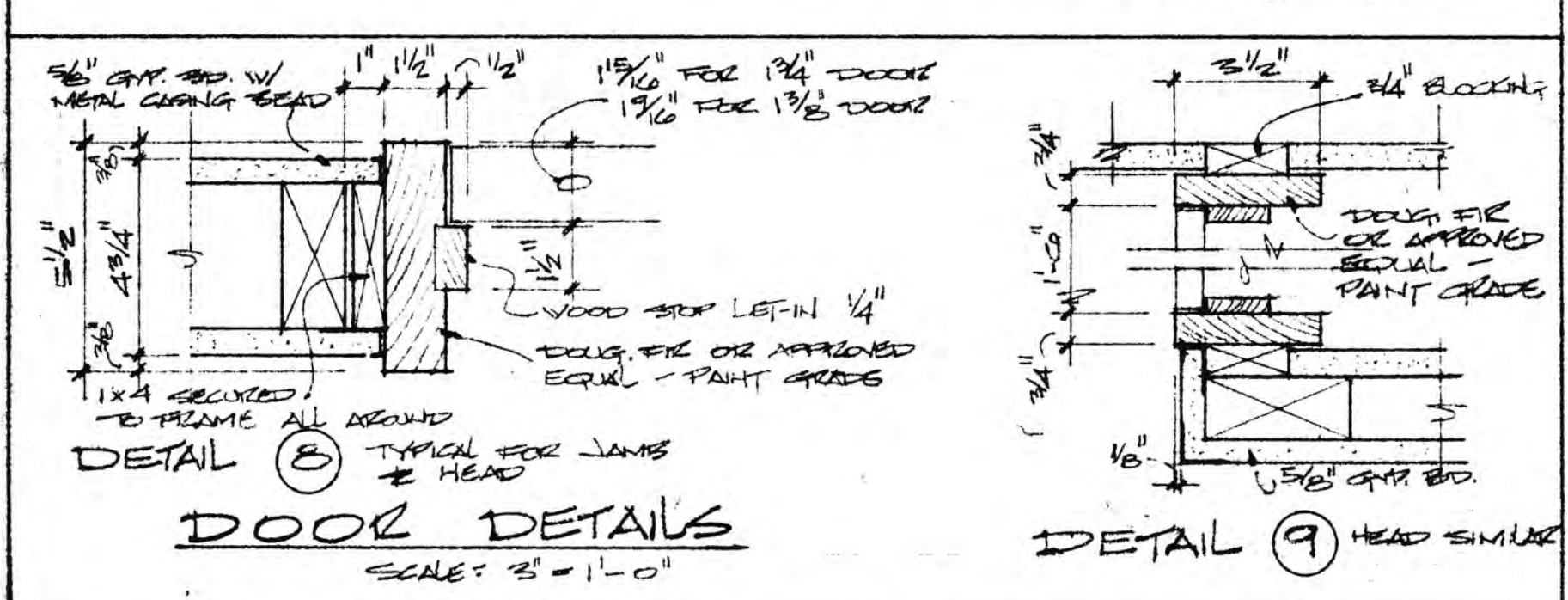
SECTION A AT ENTRY STAIRS
 SCALE: 1/2" = 1'-0"



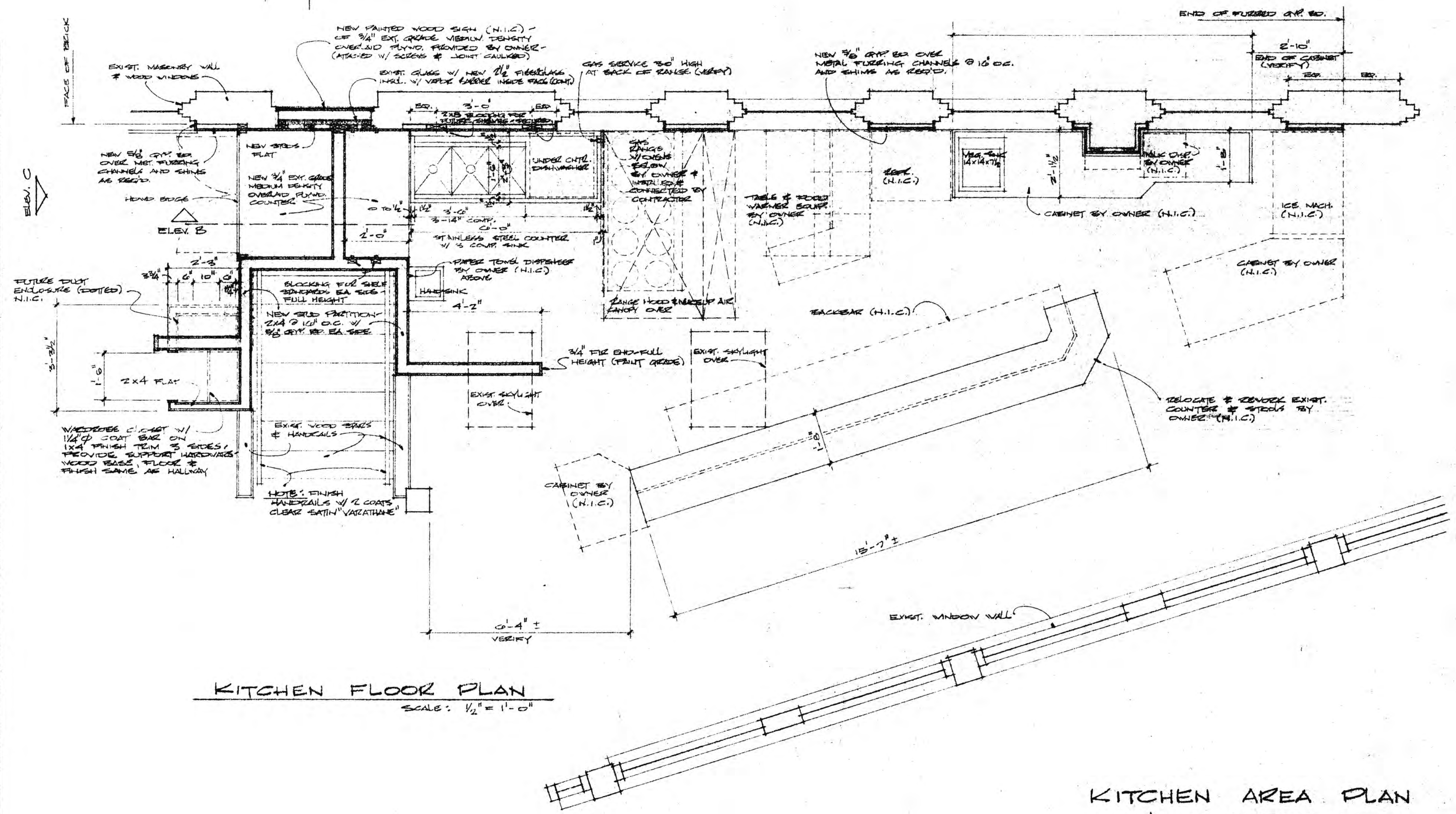
EAST KITCHEN ELEVATION (B)
 SCALE: 1/2" = 1'-0"



CABINET DETAILS
 SCALE: 1/2" = 1'-0"



DOOR DETAILS
 SCALE: 3/8" = 1'-0"



KITCHEN FLOOR PLAN
 SCALE: 1/2" = 1'-0"

KITCHEN AREA PLAN
 & ELEVATIONS

SPECIFICATIONS

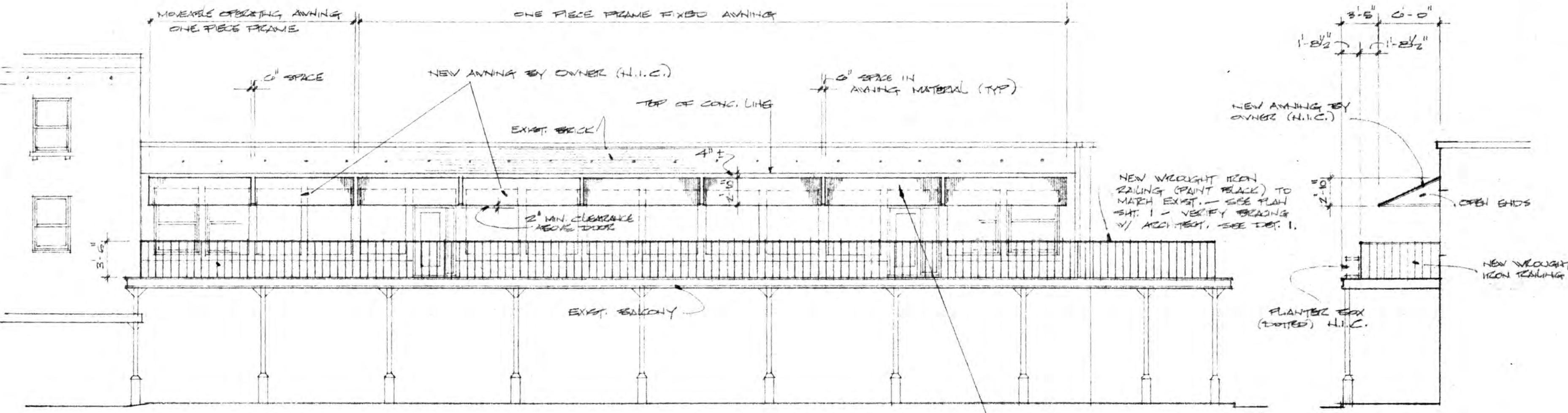
GENERAL CONDITIONS

- 1. STATEMENT OF WORK**
This contract includes all labor, materials, and tools for the completion of the work shown on the plans, stated in the specs, or reasonably implied. Items indicated N.I.C. or by Owner are not in this contract.
- 2. INSTRUCTIONS TO BIDDERS**
All contractors and subcontractors bidding this work shall visit the premises and shall inform themselves of existing conditions.
- 3. BUILDING CODE**
Requirements of the local building codes and/or ordinances shall be strictly complied with as well as all other state and national regulations. Contractor shall obtain all permits and inspections.
- 4. OLD WORK**
Work remaining in place that is damaged by reason of work under this contract shall be restored to its condition at the time of award of contract. If removal of existing work discloses unfinished or untrue surfaces, such surfaces shall be replaced or refinished to make work uniform, true and finished.
- 5. CLEAN UP**
After all trades have completed their work, the Contractor shall:
a. Vacuum entire interior of project.
b. Hand dust and clean shelves and cabinets.
c. Clean finish floors and floor covering.
d. Leave entire project perfectly clean and suitable for occupancy.
- 6. SUBSTITUTIONS**
Manufacturer and catalog number of items indicated in drawings and specifications establish design and quality desired. If substitutions are suggested by contractor, approval is required by the Architect.
- 7. GUARANTEE OF WORK**
Contractor shall guarantee all materials and workmanship for one year after acceptance by owner.

GENERAL WORK

- A.1. PARTITIONS**
Partitions shall be wood stud (min 2x4) or metal stud (min 3-5/8") as shown on drawings unless otherwise indicated, constructed to ceiling. Wood framing shall be construction grade minimum grade marked. Metal studs and framing shall be U.S. Gypsum or equal. Partitions, framing, and ceilings shall be finished with 5/8" gypsum wall board Type X with approved fastening. Finish smooth and clean with "Fast-A-Tape" joint reinforcement and concealment ready for paint. Space framing 16" o.c. Provide metal casing head at ends, joints abutting dissimilar materials, and door frames for gypsum board construction. Provide metal corner bead all outside corners.
- A.2. ADDITIONAL FINISHES**
Provide sound absorption treatment at partitions shown on drawings. Sound absorption material: 1" U.S. Gypsum Fiberboard rock wool 0.175 lbs./sq. ft. Attach to wall studs with fasteners per manufacturer's instructions.
Acoustical sealant: U.S. Gypsum acoustical sealant all around or approved non-setting, non-staining, acoustically tested sealant.
- A.3. TOILET PARTITIONS**
Toilet partitions shall be metal based enamel finish "Embacry" line with headrest bracket, by Elwood Special Products, as approved equal. Provide blocking secured in wall for installer. Provide and install all standard fittings, washers, and door hardware by manufacturer to complete installation. Install rigid, plumb and level, in accordance with layout on drawings. Provide concealed latch with emergency feature, and combination sport hook and bumper No. 150 on door. Clearance at vertical edges of doors uniform, not to exceed 3/16". Colors selected by architect from standard colors available. Verify dimensions in field and furnish shop drawings to Architect for approval before installation.
- A.4. WOOD DOORS**
Provide wood doors as shown on drawings to conform to National Woodwork Manufacturers Association (NWMA) Interior Standard: I.S. 1-73 Hardwood Veneered Flush Door and grade standards of Architectural Woodwork Institute. Finish shall be paint grade. Finish (poor grade) painted, with primer and enamel finish to cover, color selected by Architect. Doors shall have "mild option" handlock edges to match.
- A.5. WASHROOM ACCESSORIES**
Provide washroom accessories and grab bars in toilets as shown on drawings. Numbers indicated are for accessories manuf. by "Bourique". Provide blocking in wall to secure. See A.5. BLOCKING.
- A.6. BLOCKING**
Provide wood blocking or other approved support in walls for anchoring items shown on drawings attached to walls, including shelving, toilet partitions, washroom accessories, mirror and wall stops.
- A.7. STAFF LOCKERS**
Provide staff lockers built-in where shown on drawings, metal "Republic" Type B Box with flat top, flat key locks and padded handles. 15 lockers (3 wide x 5 high) each 12" wide x 12" deep x 24-1/2" high overall 3'-1 1/4" wide x 72" high. Build on platform from floor to base height.
- A.8. PAINTING**
Paint prime and finish all walls and ceilings and areas as called out on drawings, including grilles and registers and exposed ductwork and conduit, with minimum two (2) coats to cover. Colors selected by Architect.

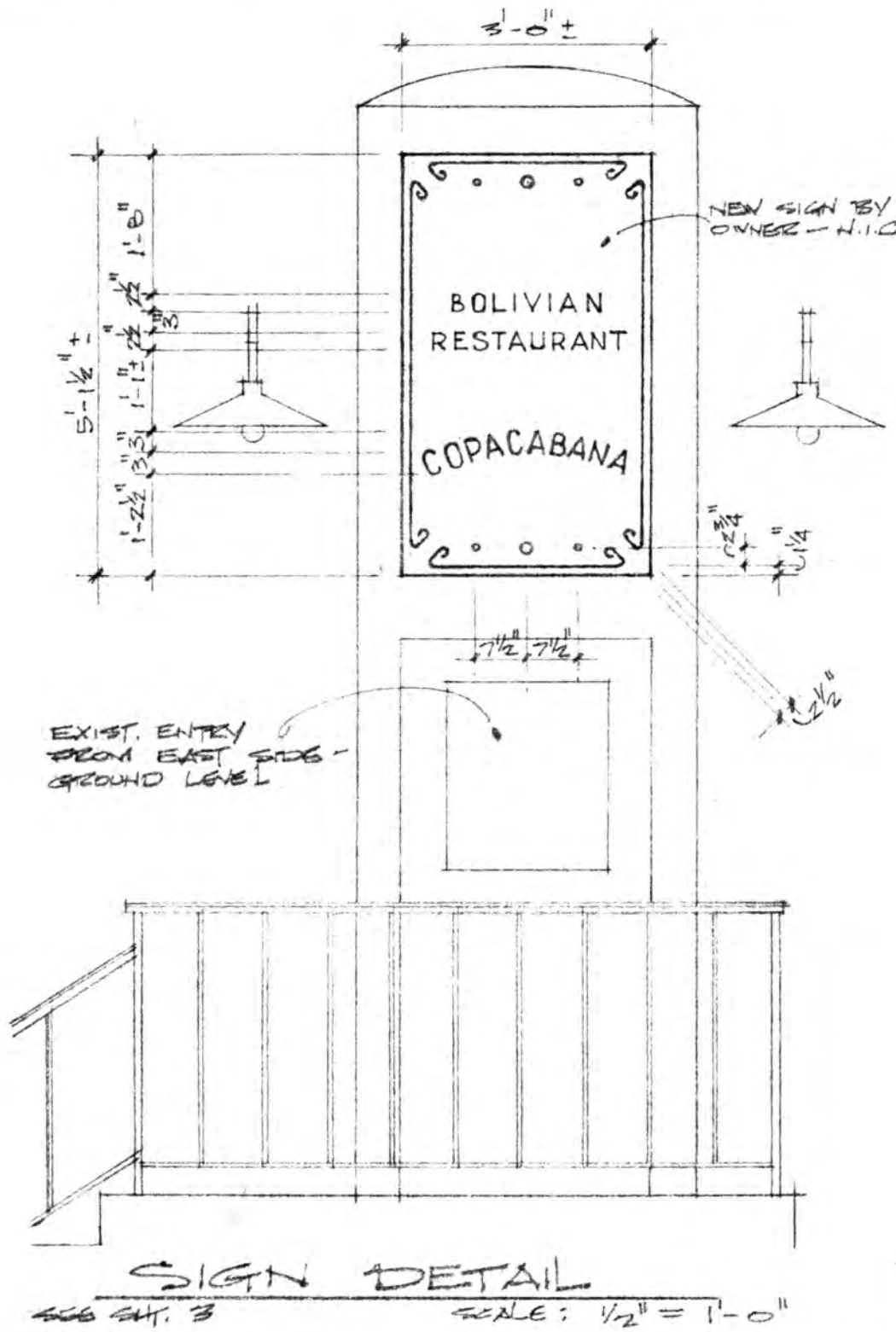
- A.9. MECHANICAL DUCTWORK**
Provide mechanical ductwork as shown on the drawings. Minor adjustments shall be made by contractor in holowness, size, configuration and location of ductwork if required by Architect to fit clearances in existing space.
- A.10. CANOPY RANGE HOOD**
Provide canopy range hood as shown on drawings. Verify dimensions in field. Provide shop drawings for approval by Architect. As seams and joints shall have liquid tight continuous external weld. Construction to conform to NFPA No. 96. Provide approved filters and 2 vapor proof lights with shatterproof design, and dry chemical extinguishing system. See specs. A.11. KITCHEN HOOD FIRE PROTECTION.
Provide and install a complete dry chemical fire extinguishing system for the protection of the kitchen canopy range hood, duct, plenum and cooking surface areas. The system shall be fully automatic and augmented with a manual control operation. Provide with automatic shutoff for gas service. Entire installation shall comply with NFPA #96 and Seattle Fire and Building Dept. requirements.
- A.12. ROOFING WORK**
All roofing work including membranes, flashing and napping work shall be done with the approval or supervision of Grove Roofing Co. (Roofing agent) and shall be part of this contract.
- A.13. PLASTER ROOF EQUIPMENT**
Paint all exposed galvanized sheet metal and exposed metal equipment on roof with blue to match existing on other roof equipment. Prepare galvanized and prime with approved methods to ensure full life to final coats. Paint minimum two (2) coats to cover.
- A.14. WATER PIPES INSULATION**
Insulate H.W. & C.W. supply pipes throughout with minimum 1" "Embater" or approved equal.
- A.15. EXTRA MATERIAL**
Contractor shall provide extra quantities of unused finish materials used in remodeling, to the owner at completion of work. Include minimum: 1 gal. ea. wall color, except 1 quart Man and Woman toilet wall paint, 1 quart ea. trim and accent color, 1 box vinyl asbestos tile ea. color.



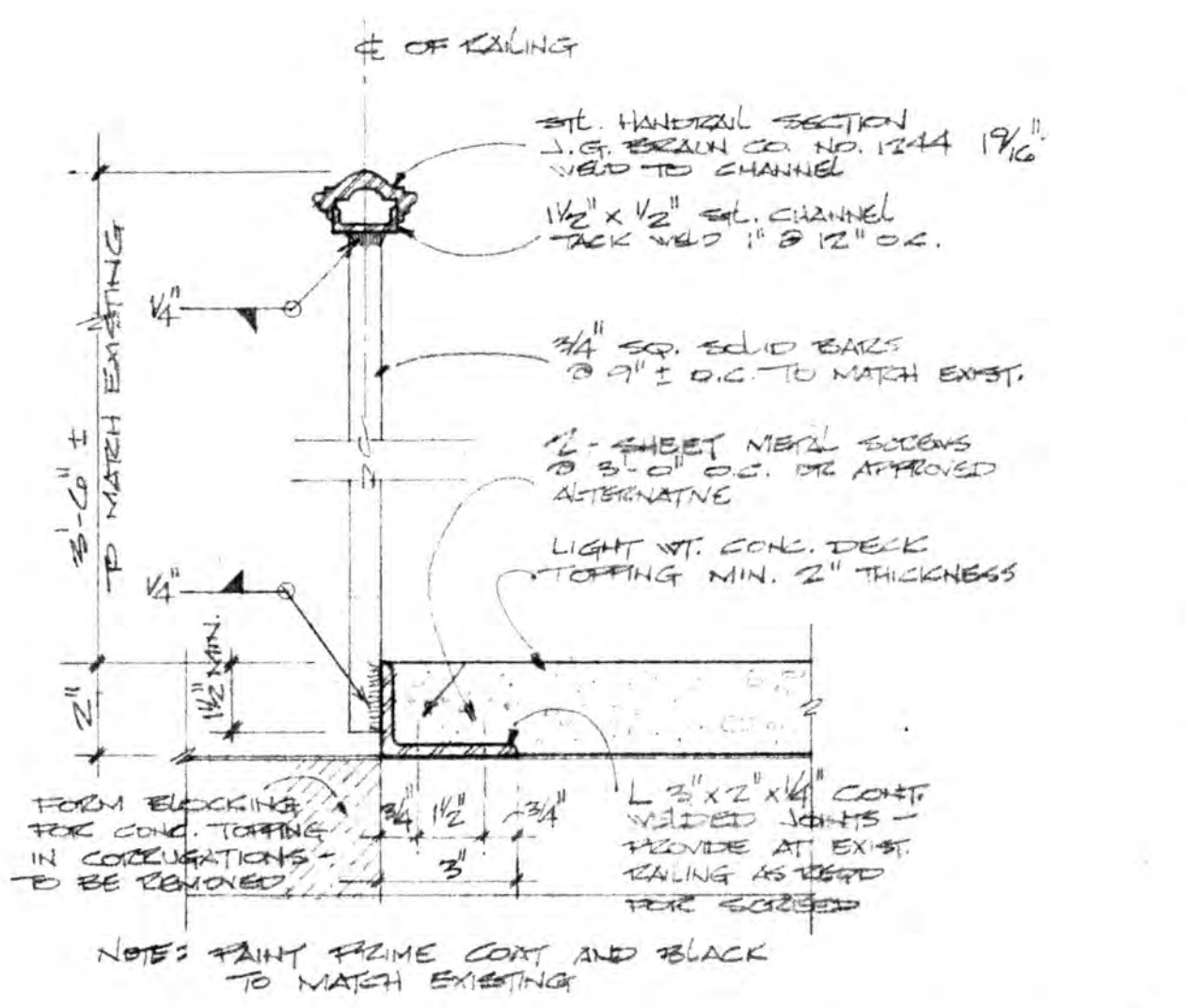
WEST ELEVATION (EXTERIOR)
SCALE: 1/8" = 1'-0"

SECTION

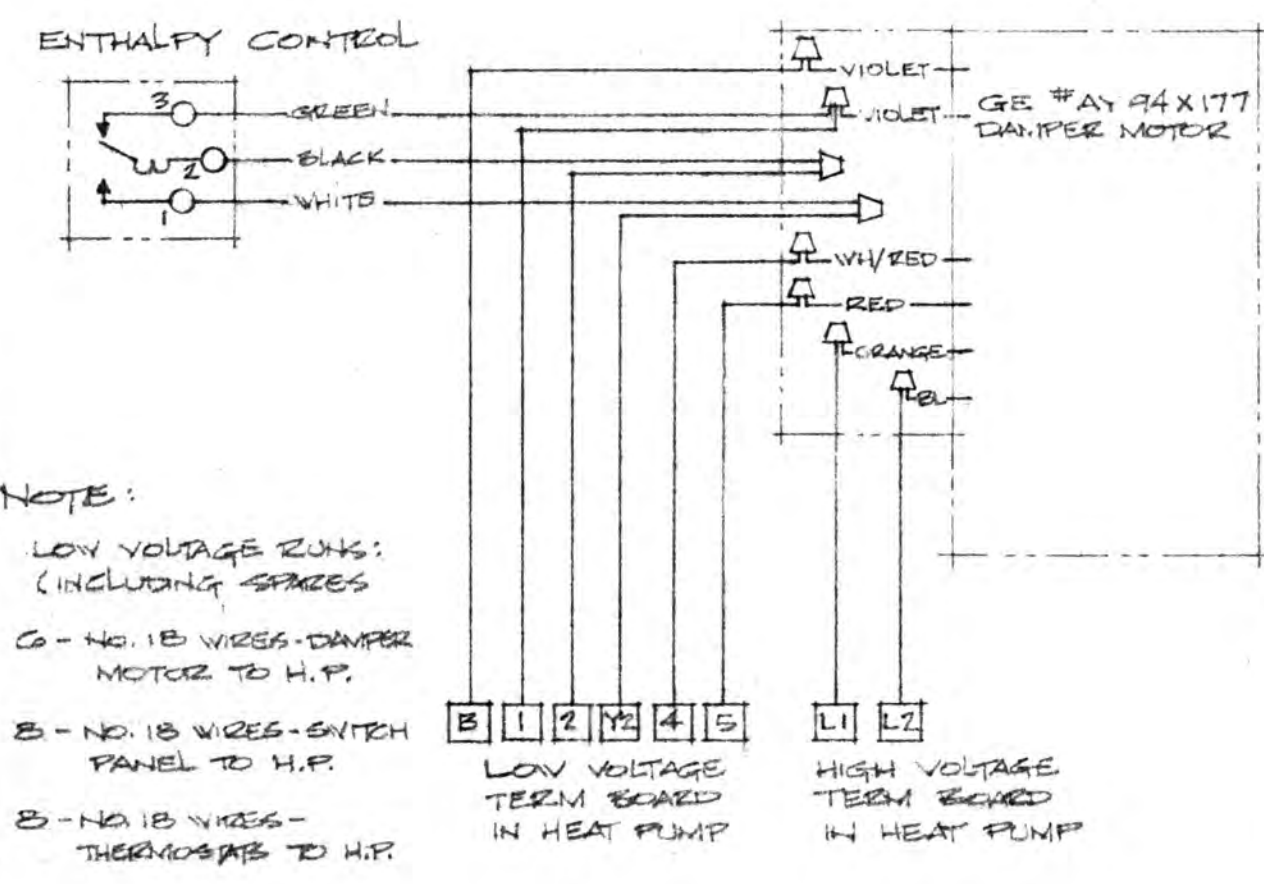
AWNING (BY OWNER - N.I.C.)
AWNING FRAMES SHALL BE 3/4" GALVANIZED PIPE, MOUNTED TO EXISTING CONCRETE HEADER BEAM WITH WOOD HEAD EXPANSION BOLTS OF APPROVED EQUAL.
FABRIC SHALL BE "PRO-TEC" 16 OZ. LAMINATE VINYL TO MEET FIRE RESISTANT STANDARDS OF THE SEATTLE BUILDING DEPARTMENT. ALL BODY SEAMS ON FABRIC SHALL BE ELECTRICALLY WELDED.
INSTALL WITH APPROVED GALVANIZED CLAMPS, CONNECTIONS, HINGES, AND OPERATING MEANS AND CHAINS.
SUBMIT SHOP DRAWINGS FOR APPROVAL BY THE ARCHITECT.
FABRIC COLOR SHALL BE 1515 BUFF



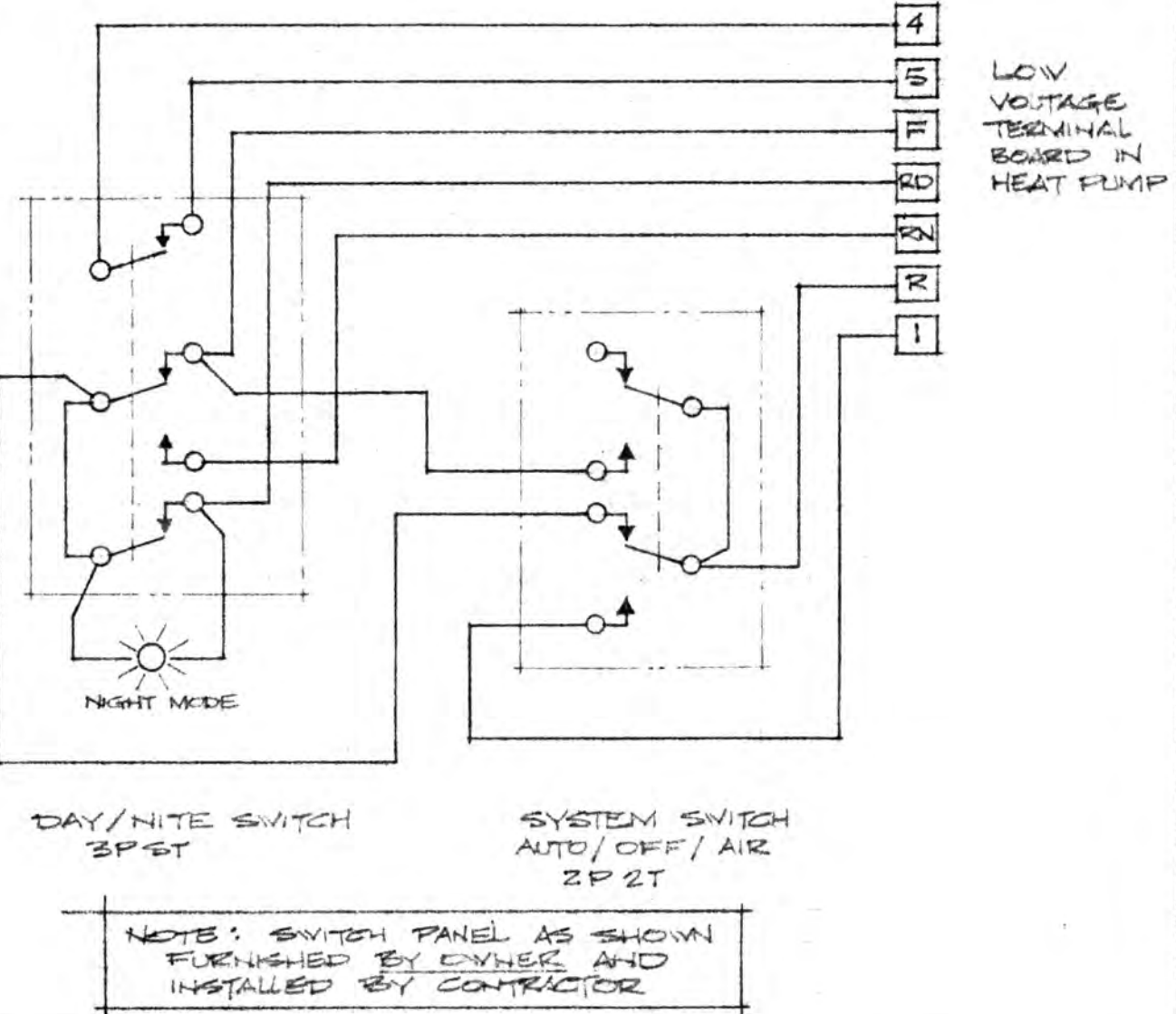
SIGN DETAIL
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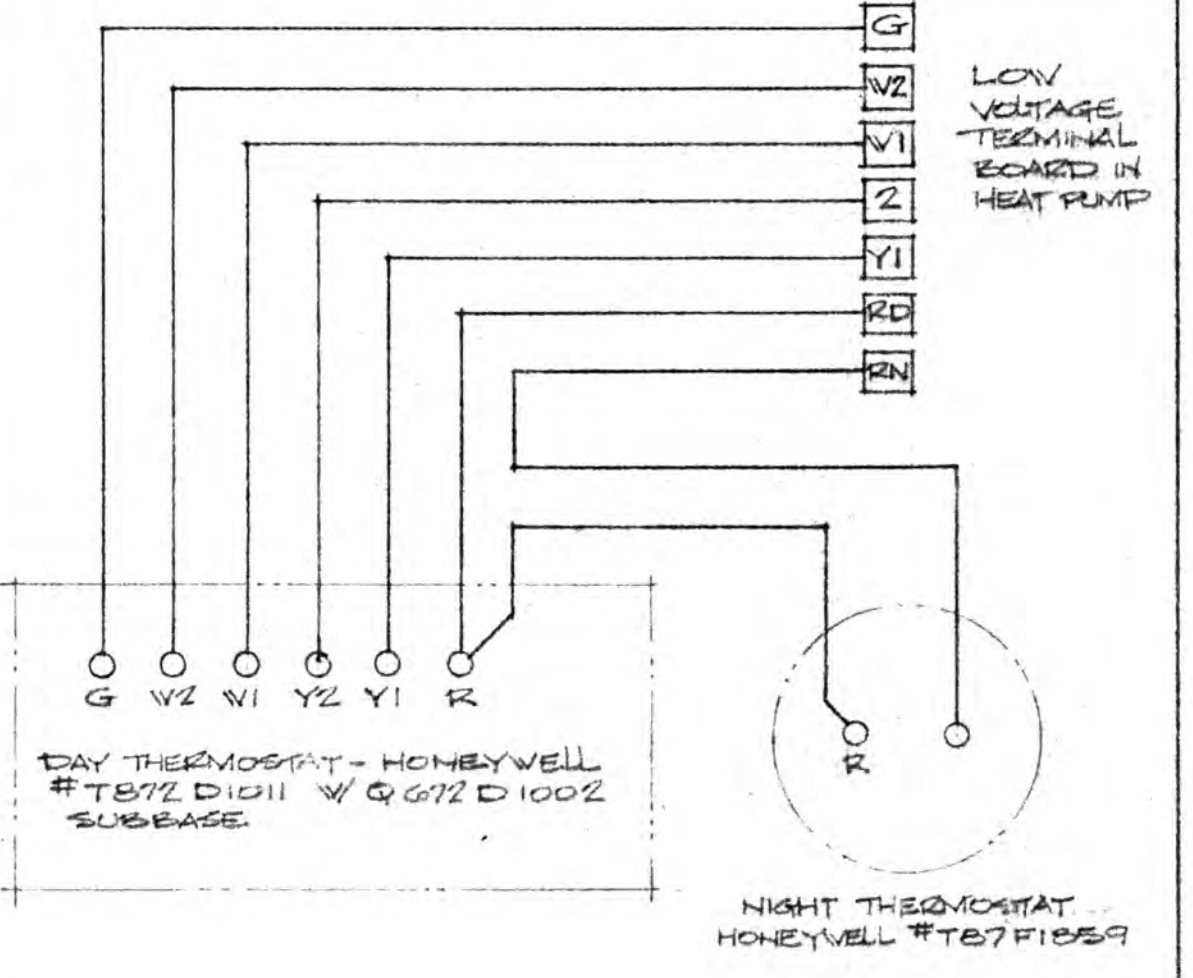
DECK TOPPING & RAILING DETAIL ①
SCALE: 3/8" = 1'-0"



WIRING DIAGRAM 1
ECONOMIZER SYSTEM



WIRING DIAGRAM 2
SWITCH PANEL



WIRING DIAGRAM 3
THERMOSTATS

SPECIFICATIONS EXTERIOR DETAILS MISCELLANEOUS